## Appendix F

## Transportation Systems Technical Report \#1 and 2

## Transportation System Technical Report

## Warden Avenue and Kennedy Road Class Environmental Assessment

York Region

# Transportation System Technical Report 

Warden Avenue and Kennedy Road Class Environmental Assessment

York Region

R.J. Burnside \& Associates Limited

6990 Creditview Road, Unit 2
Mississauga ON L5N 8R9 CANADA

June 6, 2022
300052314.0000

Transportation System Technical Report June 6, 2022

## Distribution List

| No. of <br> Hard <br> Copies | PDF | Email | Organization Name |
| :---: | :---: | :---: | :---: | :---: |
| 0 | Yes | Yes | York Region |

## Record of Revisions

| Revision | Date | Description |
| :---: | :---: | :--- |
| 0 | December 9, 2021 | Initial Submission to York Region |
| 1 | April 6, 2022 | Second Submission to York Region |
| 2 | May 20, 2022 | Third Submission to York Region |
| 3 | June 6, 2022 | Final Submission to York Region |

## R.J. Burnside \& Associates Limited

Report Prepared By:


Senior Transportation Planner
GH:lam

Report Reviewed By:


Ray Bacquie, P.Eng.
Project Director

## Executive Summary

## Introduction

The Regional Municipality of York ("York Region") retained R.J Burnside \& Associates Ltd. ("Burnside") to complete a Municipal Class Environmental Assessment (MCEA) for two study corridors: Warden Avenue and Kennedy Road between Major Mackenzie Drive East and Elgin Mills Road East. York Region has completed the Region-wide Transportation Master Plan (TMP) in 2016. As part of the TMP, the road needs and justifications were established for the two study corridors meeting the requirements for Phase 1 and 2 of the MCEA process. This report documents the analysis methodology, results of the transportation assessment and proposed road network improvements to accommodate future demand along the study corridors. Burnside confirmed and built upon the findings of York Region's 2016 TMP.

## Planning Context

A review of Provincial, York Region, and City of Markham guiding documents and policies were undertaken to establish the planning context. The 2016 TMP identified Warden Avenue and Kennedy Road as requiring roadway improvements. The 2016 TMP recommended Kennedy Road between Major Mackenzie Drive to Donald Cousens Parkway (Project ID 2003) to widen from two to four lanes and to be constructed to an urban arterial standard. The justification provided was that the transportation forecast meets threshold for a 4-lane widening. This recommendation provided an opportunity to improve walking and cycling facilities. The 2016 TMP recommended road improvements from Major Mackenzie Drive East to Elgin Mills Road East by 2031.

The 2016 TMP recommended Warden Avenue between Major Mackenzie Drive to Donald Cousens Parkway (Project ID 2073) to be widened from two to four lanes and to be constructed to an urban arterial standard. The justification provided was that the transportation forecast meets threshold for a 4-lane widening. This recommendation provided an opportunity to improve walking and cycling facilities. The 2016 TMP recommended the road improvements from Major Mackenzie Drive East to Elgin Mills Road East by 2031.

Around the two study corridors, the City of Markham's Official Plan has identified 1,300 hectares of lands, which includes 975 hectares of developable land, in North Markham to be the 'Future Urban Area'. Approximately 700 hectares of the developable land are designated 'Future Neighbourhood Area', which will accommodate approximately 45,000 persons by full build-out. Approximately 275 hectares of developable land north of Elgin Mills Road are designated as 'Future Employment Area', which will accommodate approximately 19,000 jobs at full build-out.

## Study Areas

There are two study corridors, which include the following road segments:

- Warden Avenue, between Major Mackenzie Drive East and Elgin Mills Road Eas.
- Kennedy Road, between Major Mackenzie Drive East and Elgin Mills Road East.

The Study Areas are illustrated below.
The Study Area for Warden Avenue also includes a short segment approximately 65 m south of Major Mackenzie Drive East to include the large culvert that carries Berczy Creek under Warden Avenue. The Study Area for Kennedy Road also include a short segment approximately 120 m north of Elgin Mills Road East to include the bridge that carries Bruce Creek under Kennedy Road. While the Study Areas have been extended to include these 2 watercourse crossings, the Study Area limits referenced for the purposes of this report are the intersections at Major Mackenzie Drive East and Elgin Mills Road East.


## Existing Transportation Conditions

A transportation system inventory was established, which documented the road network, transit network, active transportation network, goods movement-related characteristics, road right-of-way characteristics, and existing driveway accesses.

## Travel Demand

Various transportation data sets provided insight on the travel patterns, mode share, and temporal distribution of traffic volumes. Analysis of 2016 Transportation Tomorrow Survey data showed that most travel to and from the area are done by driving or being a passenger of a private automobile. Historic traffic counts showed that for both Warden

Avenue and Kennedy Road, the southbound direction is the peak direction in the AM peak hour. The northbound direction is the peak direction in the PM peak hour.

The traffic volumes for the existing conditions during AM and PM peak hours were derived and normalized from various data sets including historical turning movement counts and Annual Average Daily Traffic (AADT).

## Existing Conditions Multi-Modal Level-of-Service Evaluation

Using York Region's Transportation Mobility Plan Guidelines (November 2016), levels-of-service were evaluated for three transportation modes: automobile, cycling, and walking. There is currently no transit service along these two study corridors.

## Future Transportation Conditions

The road and transit network were documented based on planned improvements from York Region’s 2016 TMP, 5-Year York Region Transit Service Plan, Regional Official Plan (2010), and the City of Markham's Active Transportation Master Plan (2021).

## Future Conditions Forecasted Traffic Volumes

Transportation model refinements were made to York Region's EMME model for the 2031 and 2041 horizon years. Traffic growth rates were calculated based on modelling outputs. The traffic growth rates were calculated using peak direction modelled volumes in the AM peak hour, which is the time period of the EMME model. These traffic growth rates were applied to both directions of the existing conditions AM and PM traffic volumes derived from the turning movement counts to forecast 2041 traffic volumes.

## Future Conditions Automobile Level-of-Service Evaluation

Level-of-service evaluation for the "Do Nothing" scenario was undertaken using the future conditions traffic volumes and the existing network with no geometric improvements. The "Do Nothing" scenario was assessed to re-confirm the automobile needs and justification that was identified in the 2016 Transportation Master Plan. The 2016 TMP fulfills the requirements of Phases 1 and 2 of the Municipal Class Environmental Assessment process for roads. This TMP identified Warden Avenue and Kennedy Road as requiring roadway improvements which included a one to two-lane widening within the Study Areas.

## Automobile Demand Needs and Justification:

In the 2021 existing conditions year:

- During the AM peak hour, the traffic operations analysis indicated three intersections operating with demand at or above capacity. Warden Avenue and Elgin Mills Road East operate at LOS E. Warden Avenue and Major Mackenzie Drive East operate at LOS F. Kennedy Road and Major Mackenzie Drive East operate at LOS E.
- During the PM peak hour, the traffic operations analysis indicated one intersection operating with demand at or above capacity. Warden Avenue and Major Mackenzie Drive East operate at LOS E.
- The link capacity analysis indicated that Warden Avenue is operating with volumes above typical auto link capacity during both AM and PM peak hours. The maximum auto link volume-to-capacity ratio was 1.24 in the southbound direction during the AM peak hour. The maximum auto link volume-to-capacity ratio was 1.15 in the northbound direction during the PM peak hour.
- This link capacity analysis indicated that Kennedy Road is operating below total link capacity in the existing conditions. The maximum link volume-to-capacity ratio is 0.78 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 0.58 in the northbound direction during the PM peak hour.

To re-confirm the findings of the Region's 2016 Transportation Master Plan, the 2041
"Do Nothing" scenario was assessed.
In the 2041 future conditions year for the "Do Nothing" scenario:

- During the AM and PM peak hours, the traffic operations analysis indicated that all four intersections are forecasted to operate with volumes above capacity at LOS F indicating a need for intersection improvements.
- In the AM peak hour, the southbound-through forecasted queue along Warden Avenue between Major Mackenzie Drive East and Elgin Mills Road East is significantly long. The recommended improvements of widening from two to four lanes to increase the link capacity will dramatically reduce the forecasted queues.
- The other movement that is forecasted to have 95th percentile through movement queues long enough to impact existing adjacent signalized intersection was westbound at Kennedy Road and Major Mackenzie Drive East. Signal timing optimization and coordination is recommended to mitigate spillback at the adjacent intersections.
- The 95th percentile queue is forecasted to be greater than the existing northbound left storage distance at Kennedy Road and Major Mackenzie Drive East. Infrastructure improvements (e.g., dual left turns), increasing left turn storage distances, signal timing optimization, or lane configuration alterations are recommended to be explored to mitigate the impacts.
- The link capacity analysis indicated that Warden Avenue is forecasted to operate above auto link capacity in the future "do nothing" scenario during the AM and PM peak hours. The maximum link volume-to-capacity ratio is 2.22 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 1.85 in the northbound direction during the PM peak hour.
- The link capacity analysis indicated that Kennedy Road is forecasted to operate above auto link capacity in the future "do nothing" scenario during the AM peak hour. The maximum link volume-to-capacity ratio is 1.26 in the southbound direction during
the AM peak hour. The maximum link volume-to-capacity ratio is 0.89 in the northbound direction during the PM peak hour.
- The 2041 future year link capacity analysis indicated the need for increased link capacity for Warden Avenue and Kennedy Road. Increased link capacity should be provided by increasing the number of lanes from one lane per direction to two lanes per direction as recommended by the Region's 2016 TMP.


## Pedestrian and Cycling Needs and Justification

- There are no existing dedicated cycling facilities along Warden Avenue or Kennedy Road within the Study Areas.
- Pedestrian Levels-of-Service did not meet the target of LOS C for many legs of intersections and roadway segments within the study corridors under the existing condition.
- Improvements to the pedestrian and cycling environment should be implemented at the intersections of Elgin Mills Road East at Warden Avenue and Kennedy Road. Improvements to the pedestrian and cycling environment should be implemented on Warden Avenue and Kennedy Road between Elgin Mills Road East and Major Mackenzie Drive East.
- Because the area is being redeveloped and reconstructed, opportunities to improve the pedestrian and cycling environment should be explored at the Major Mackenzie Drive East intersections at Warden Avenue and Kennedy Road concurrently with the improvements to the rest of the study corridors to ensure consistency and connectivity.
- There is an existing multi-use path along Warden Avenue between 16th Avenue and Major Mackenzie Drive East on the east side. Given that the area is planned for redevelopment with an urban cross section for Warden Avenue, it would be appropriate to continue the multi-use path north of Major Mackenzie Drive East on the east side for connectivity of the active transportation network.


## Transit Needs and Justification

- There is no transit service currently provided within the Study Areas. The Region supports transit as a robust transit network helps support growth to key centres and corridors and minimizes the need for travel and reduces dependence on single occupant vehicles. This sustainable mode of transportation also helps mitigate climate change, which is another important objective for York Region.
- Warden Avenue and Kennedy Road between Elgin Mills Road East and Major Mackenzie Drive East are planned to be Frequent Transit Network (FTN) routes.


## Traffic Safety Needs and Justification

- A safety assessment was undertaken to identify and mitigate potential safety related concerns using historical collision records within the Study Areas.
- Warden Avenue between Major Mackenzie Drive East and Heritage Hill Drive has collision rate of 1.80 along this segment indicating a need for safety improvements.
- At the Kennedy Road and Elgin Mills Road East intersection, 47\% of the collisions were categorized as non-fatal and $53 \%$ was categorized as property damage only. The severity of the collisions may represent an opportunity for improvement such as speed management.
- The alternative design concepts should consider the need to protect for vulnerable road users since active transportation activity is expected to increase along this corridor.


## Roundabout Needs and Justification

- This study does not recommend including the implementation of roundabouts as an alternative to be considered for both study corridors at the Major Mackenzie Drive East and Elgin Mills Road East intersections based on the following reasons:
- Given the forecasted 2041 traffic volumes, a 3-lane roundabout would have to be considered. A 3-lane roundabout is not recommended in York Region currently, and specifically in the Future Urban Area where in an urbanized environment, comfort for vulnerable road users and those that have mobility challenges should be prioritized.
- Roundabouts should be considered if there are safety concerns at the intersections. However, these intersections are not shown to have safety problems as shown in the traffic safety assessment. Although single-lane roundabouts can improve safety, a multi-lane roundabout may not have the same level of safety benefits.
- The four intersections within the study corridors are already signalized and generally roundabout implementation should be considered for new or unsignalized intersections.


## Alternative Solutions

York Region's Transportation Master Plan (TMP) is the Region's long-term vision for York Region's transportation network, encompassing strategy, policy, and infrastructure. The plan looks ahead 30 years and considers the Region's transportation infrastructure needs to support growth and the changing needs of travellers. The plan supports healthy communities and economic growth by planning for safe, reliable travel, and efficient movement of goods.

The TMP was last conducted in 2016 and satisfies Phases 1 and 2 of the MCEA process. The TMP identified Warden Avenue and Kennedy Road as requiring roadway improvements. These recommendations were based on assessments of alternative scenarios. A preferred scenario was selected based on its alignment with the following TMP objectives:

- Support Transit.
- Support Road Network.
- Support Active Transportation.
- Support Goods Movement.
- Support Last Mile.

Transportation System Technical Report
June 6, 2022
The preferred scenario for Warden Avenue included the widening of the corridor to 4 lanes and constructing to an urban arterial standard. The justification provided was that the traffic forecast met threshold for a 4-lane widening. This recommendation provided an opportunity to improve walking and cycling facilities. The TMP recommended the widening from Major Mackenzie Drive East to Elgin Mills Road East by 2031 .

The preferred scenario for Kennedy Road included the widening of the corridor from two lanes to four lanes and constructing to an urban arterial standard. The justification provided was that the traffic forecast met threshold for a 4-lane widening. This recommendation provided an opportunity to improve walking and cycling facilities. The TMP recommended the widening from Major Mackenzie Drive East to Elgin Mills Road East by 2031.

## Table of Contents

1.0 Introduction ..... 1
1.1 Background ..... 1
1.2 Report Structure ..... 1
1.3 Description of the Study Areas ..... 1
1.4 Transportation System Study Approach ..... 2
2.0 Planning Context ..... 3
2.1 Provincial Planning Context ..... 3
2.1.1 Provincial Policy Statement, 2020 ..... 3
2.1.2 Places to Grow ..... 3
2.1.3 Greenbelt Plan ..... 4
2.1.4 Oak Ridges Moraine Conservation Plan ..... 5
2.2 York Region Planning Context and Related Studies ..... 5
2.2.1 York Region Official Plan ..... 5
2.2.2 York Region Strategic Vision (Vision 2051), 2011 ..... 5
2.2.3 York Region's Sustainability Strategy ..... 6
2.2.4 York Region Transportation Master Plan ..... 6
2.2.5 York Region's Pedestrian and Cycling Master Plan (2007) ..... 7
2.2.6 York Region Transit Business Plan (2021 - 2025) ..... 8
2.2.7 York Region's 2019 Designing Great Regional Streets ..... 8
2.3 City of Markham Planning Policies and Related Studies ..... 9
2.3.1 City of Markham Official Plan ..... 9
2.3.2 Future Urban Area Conceptual Master Plan ..... 9
2.3.3 Markham Active Transportation Master Plan ('Ride \& Stride’) (2021) ..... 10
2.3.4 Future Urban Area Collector Road Network Class Environmental Assessments ..... 11
2.3.5 Elgin Mills Road Municipal Class Environmental Assessment. ..... 12
3.0 Data Collection ..... 13
3.1 Turning Movement Counts ..... 13
3.2 Supplemental Traffic Studies ..... 13
3.3 Annual Average Daily Traffic ..... 14
3.4 Existing Signal Timing Plans ..... 14
3.5 Speed Studies ..... 14
3.6 Collision Data ..... 14
3.7 York Region Transportation Forecasting Model. ..... 14
4.0 Existing Transportation Conditions ..... 15
4.1 Transportation System Inventory ..... 15
4.1.1 Road Network ..... 15
4.1.2 Transit Network ..... 16
4.1.3 Active Transportation Network ..... 17
4.1.4 Goods Movement ..... 17
4.1.5 Road Right-of-Way Characteristics ..... 18
4.1.6 Existing Accesses ..... 18
4.2 Travel Demand ..... 19
4.2.1 Travel Patterns ..... 19
4.2.2 Mode Share ..... 21
4.2.3 Temporal Distribution of Traffic Volumes ..... 22
4.2.4 Traffic Speed Distribution ..... 23
4.2.5 Pedestrian Activity ..... 25
4.2.6 Existing Conditions Traffic Volumes ..... 25
4.3 Automobile Level of Service Evaluation ..... 27
4.3.1 Existing Link Capacity Analysis ..... 27
4.3.2 Existing Traffic Operations ..... 29
4.3.3 Existing Intersection Queues ..... 33
4.4 Pedestrian Level of Service Evaluation. ..... 35
4.5 Cycling Level of Service Evaluation ..... 37
4.6 Transit Level of Service Evaluation ..... 38
4.7 Traffic Safety ..... 38
4.7.1 Review of Collision Data ..... 38
4.7.2 Collision Rates ..... 38
4.7.3 Collisions by Severity ..... 41
4.7.4 Collisions by Initial Impact Type ..... 42
4.7.5 Collisions by Vulnerable Road Users ..... 48
4.7.6 Collisions by Year, Month, and Day of the Week ..... 48
4.7.7 Collisions by Environmental Conditions ..... 50
4.7.8 Collision Analysis for Warden Avenue between Major Mackenzie Drive East and Heritage Hill Drive ..... 51
5.0 Future Transportation Conditions ..... 52
5.1 Future Land Use ..... 52
5.2 Planned Road Network ..... 53
5.3 Planned Transit Service ..... 55
5.4 2041 Future Automobile Demand ..... 55
5.4.1 Transportation Model Refinements ..... 55
5.4.2 Traffic Growth Rates ..... 57
5.4.3 2041 Traffic Volumes ..... 60
5.52041 "Do Nothing" Automobile Level of Service Evaluation ..... 61
5.5.1 2041 Link Capacity Analysis ..... 63
5.5.2 2041 Traffic Operations - "Do Nothing" Scenario ..... 64
5.5.3 2041 Future Intersection Queues - "Do Nothing" Scenario ..... 67
6.0 Signal Warrant Analysis ..... 69
7.0 Roundabout Screening ..... 71
8.0 Summary of Needs and Justifications ..... 73
9.0 Alternative Solutions ..... 76
9.1 Overview ..... 76
9.2 Warden Avenue between Major Mackenzie Drive and Donald Cousens Parkway (Project ID 2073) ..... 76
9.3 Kennedy Road between Major Mackenzie Drive and Donald Cousens Parkway (Project ID 2003) ..... 77
Tables
Table 1: Turning Movement Counts Provided by York Region ..... 13
Table 2: Supplemental Traffic Data Details. ..... 13
Table 3: Road Characteristics within or near the Study Area ..... 15
Table 4: All-Day Trip Productions from Traffic Analysis Zones Adjacent to the Study Corridors* ..... 20
Table 5: All-Day Trip Attractions to Traffic Analysis Zones Adjacent to the Study Corridors* ..... 21
Table 6: Mode Share for Trip Productions* ..... 21
Table 7: Mode Share for Trip Attractions* ..... 21
Table 8: Speed Attributes along Kennedy Road ..... 24
Table 9: Speed Attributes along Warden Avenue ..... 24
Table 10: Warden Avenue - Historic Average Annual Daily Traffic ..... 25
Table 11: Kennedy Road - Historic Average Annual Daily Traffic ..... 26
Table 12: Link Capacity Analysis for Warden Avenue - AM Existing Conditions ..... 28
Table 13: Link Capacity Analysis of Warden Avenue - PM Existing Conditions ..... 28
Table 14: Link Capacity Analysis for Kennedy Road - AM Existing Conditions ..... 28
Table 15: Link Capacity Analysis of Kennedy Road - PM Existing Conditions ..... 29
Table 16: Existing Conditions Traffic Operations Analysis. ..... 31
Table 17: Existing Conditions Queue Summary ..... 34
Table 18: Pedestrian Level-of-Service Evaluation ..... 35
Table 19: Cycling Level-of-Service Evaluation ..... 37
Table 20: Segment Collision Rates (2015-2019) ..... 40
Table 21: Intersection Collision Rates (2015-2019) ..... 40
Table 22: Warden Avenue - Collisions by Severity (2015-2019) ..... 42
Table 23: Kennedy Road - Collisions by Severity (2015-2019) ..... 42
Table 24: Warden Avenue - Collisions by Initial Impact Type (2015-2019) ..... 46
Table 25: Kennedy Road - Collisions by Initial Impact Type (2015-2019) ..... 47
Table 26: Warden Avenue - Collisions by Severity and Year (2015-2019) ..... 48
Table 27: Warden Avenue - Collisions by Severity and Month (2015-2019) ..... 49
Table 28: Warden Avenue - Collisions by Severity and Day of the Week (2015-2019) ..... 49
Table 29: Kennedy Road - Collisions by Severity and Year (2015-2019) ..... 49
Table 30: Kennedy Road - Collisions by Severity and Month (2015-2019) ..... 50
Table 31: Kennedy Road - Collisions by Severity and Day of the Week (2015-2019) 50
Table 32: Warden Avenue - Collisions by Environmental Conditions (2015-2019). ..... 50
Table 33: Kennedy Road - Collisions by Environmental Conditions (2015 - 2019). ..... 51
Table 34: Warden Avenue between Major Mackenzie Drive East and Heritage Hill Drive by Impact and Severity (2015-2019) ..... 51
Table 35: Population Forecasts near the Study Areas. ..... 52
Table 36: Employment Forecasts near the Study Areas ..... 52
Table 37: Modelled Volumes for the 2016 to 2031 NB/SB Traffic Growth Rate ..... 58
Table 38: Modelled Volumes for the 2016 to 2031 EB/WB Traffic Growth Rate ..... 59
Table 39: Modelled Traffic Volumes for the 2031 to 2041 NB/SB Traffic Growth Rate. ..... 59
Table 40: 2031 and 2041 AM Modelled Traffic Volumes approaching Southbound Elgin Mills Road East at Warden Avenue and Kennedy Road. ..... 60
Table 41: Link Capacity Analysis for Warden Avenue - AM Future Conditions ..... 63
Table 42: Link Capacity Analysis of Warden Avenue - PM Future Conditions ..... 63
Table 43: Link Capacity Analysis for Kennedy Road - AM Future Conditions ..... 63
Table 44: Link Capacity Analysis of Kennedy Road - PM Future Conditions. ..... 64
Table 45: Future Conditions Traffic Operations Analysis ..... 65
Table 46: Future Conditions Queue Summary ..... 67
Table 47: Alternative Scenario Analysis for Warden Avenue (2016 York Region TMP) 77
Table 48: Alternative Scenario Analysis for Kennedy Road (2016 York Region TMP).. 78
Figures
Figure 1: Study Areas ..... 2
Figure 2: Greenbelt Plan Areas ..... 4
Figure 3: Residential Blocks within the FUA ..... 10
Figure 4: Proposed Active Transportation Network (City of Markham Active Transportation Master Plan). ..... 11
Figure 5: Elgin Mills Road East Class EA Study Area. ..... 12
Figure 6: Roadway Configuration. ..... 16
Figure 7: TTC and YRT Bus Routes Near the Study Area ..... 17
Figure 8: Properties with Driveway Accesses Along Warden Avenue ..... 19
Figure 9: Properties with Driveway Accesses Along Kennedy Road ..... 19
Figure 10: Traffic Analysis Zones Adjacent to the Study Corridors. ..... 20
Figure 11: Temporal Distribution of Traffic Volumes for Kennedy Road ..... 22
Figure 12: Temporal Distribution of Traffic Volumes for Warden Avenue ..... 23
Figure 13: Traffic Volumes by Speed along Kennedy Road ..... 23
Figure 14: Traffic Volumes by Speed along Warden Avenue ..... 24
Figure 15: Existing Balanced Traffic Volumes (2021) ..... 27
Figure 16: Summary of Existing Conditions Traffic Operations Analysis ..... 32
Figure 17: Summary of Intersection and Segment Collision Rates ..... 39
Figure 18: Warden Avenue and Kennedy Road - Collisions by Severity (2015-2019) 41
Figure 19: Warden Avenue and Kennedy Road - Collisions by Initial Impact Type (2015

- 2019) ..... 43
Figure 20: Traffic Analysis Zones near the Study Areas ..... 52
Figure 21: Proposed Community Structure Plan (Conceptual Master Plan) ..... 54
Figure 22: Proposed Signal and Intersection Configuration ..... 54
Figure 23: 2031 Modelled Volumes Before and After Refinements ..... 56
Figure 24: 2041 Modelled Volumes Before and After Refinements ..... 57
Figure 25: 2041 Forecasted Traffic Volumes ..... 61
Figure 26: 2041 "Do Nothing" Lane Configuration and Traffic Control ..... 62
Figure 27: Summary of 2041 Future Conditions Traffic Operations Analysis ..... 66
Figure 28: FUA Collector Road Network ..... 69


## Appendices

Appendix A Turning Movement Counts and Supplemental Traffic Data
Appendix B Signal Timing Plans
Appendix C Speed Studies
Appendix D Collision Data
Appendix E 2021 Existing Conditions Synchro Reports
Appendix F York Region's Transportation Model Refinements EMME Plots
Appendix G 2041 Future Conditions Synchro Reports

Transportation System Technical Report
June 6, 2022

## Disclaimer

Other than by the addressee, copying or distribution of this document, in whole or in part, is not permitted without the express written consent of R.J. Burnside \& Associates Limited.

### 1.0 Introduction

### 1.1 Background

The Regional Municipality of York ("York Region") retained R.J Burnside \& Associates Ltd. ("Burnside") to complete a Municipal Class Environmental Assessment (MCEA) for two study corridors: Warden Avenue and Kennedy Road between Major Mackenzie Drive East and Elgin Mills Road East. York Region conducted their Transportation Master Plan (TMP) in 2016. As part of the 2016 TMP, the road needs and justifications were established for the two study corridors meeting the requirements for Phase 1 and 2 of the MCEA process.

### 1.2 Report Structure

As part of this Environmental Assessment, a transportation assessment was completed to assess existing and future traffic operations and safety of the study corridors to identify any operational constraints and potential safety related concerns. This report documents the analysis methodology, results of the transportation assessment, alternative solutions, and proposed road network improvements to accommodate future demand along the study corridor. Burnside confirmed and built upon the findings of York Region's 2016 TMP.

This report will be updated in Phase 3 of the MCEA process which will inform the overall evaluation of alternative design concepts for the two study corridors.

### 1.3 Description of the Study Areas

There are two study corridors, which include the following road segments:

- Warden Avenue, between Major Mackenzie Drive East and Elgin Mills Road East.
- Kennedy Road, between Major Mackenzie Drive East and Elgin Mills Road East.

The Study Area for Warden Avenue also includes a short segment approximately 65 m south of Major Mackenzie Drive East to include the large culvert that carries Berczy Creek under Warden Avenue. The Study Area for Kennedy Road also include a short segment approximately 120 m north of Elgin Mills Road East to include the bridge that carries Bruce Creek under Kennedy Road. While the Study Areas have been extended to include these two watercourse crossings, the Study Area limits referenced for the purposes of this report are the intersections at Major Mackenzie Drive East and Elgin Mills Road East.

The Study Areas are shown in Figure 1.

Transportation System Technical Report
June 6, 2022

Figure 1: Study Areas


### 1.4 Transportation System Study Approach

This study provides a preliminary assessment of the key transportation related issues, including a review of all relevant background reports / studies and existing traffic data.

An evaluation of the existing traffic operations in the area help identify opportunities to improve traffic operations. This provides the Region an opportunity to:

- Review road and access options for improvements.
- Facilitate an improved active transportation network for pedestrians and cyclists.
- Provide a multi-modal facility that is safe and efficient and can be shared by all modes of travel.

Future operations are assessed to accommodate growth through 2041 along the Warden Avenue and Kennedy Road corridors to determine the transportation needs. This can include but is not limited to auxiliary lanes, widening of the road, transportation demand management, transit, and the accommodation of active transportation infrastructure.

The Region's November 2016 Transportation Mobility Plan Guidelines were taken into consideration.

### 2.0 Planning Context

### 2.1 Provincial Planning Context

### 2.1.1 Provincial Policy Statement, 2020

The Provincial Policy Statement provides direction on land use planning, efficient development patterns, and developing strong, liveable, and healthy communities. The policies related to transportation system planning include:

- Healthy, active communities should be promoted by planning public streets, spaces, and facilities to be safe, meet the needs of pedestrians, foster social interaction, and facilitate active transportation and community connectivity.
- Transportation systems should be provided, which are safe, energy efficient, facilitate the movement of people and goods, and are appropriate to address projected needs.
- Efficient use should be made of existing and planned infrastructure, including through the use of transportation demand management strategies, where feasible.
- As part of a multimodal transportation system, connectivity within and among transportation systems and modes should be maintained and, where possible, improved including connections, which cross jurisdictional boundaries.
- A land use pattern, density and mix of uses should be promoted that minimize the length and number of vehicle trips and support current and future use of transit and active transportation.
- Planning authorities shall plan for and protect corridors and rights-of-way for infrastructure, including transportation, transit and electricity generation facilities and transmission systems to meet current and projected needs.
- Major goods movement facilities and corridors shall be protected for the long term.
- Planning authorities shall support energy conservation and efficiency, improved air quality, reduced greenhouse gas emissions, and preparing for the impacts of a changing climate through land use and development patterns, which promote the use of active transportation and transit in and between residential, employment (including commercial and industrial) and institutional uses and other areas.


### 2.1.2 Places to Grow

A Place to Grow ("Growth Plan") contains policies and schedules to plan for growth and development in a way that supports economic prosperity, protects the environment, and helps communities achieve a high quality of life. This Growth Plan also sets out a long-term framework for managing growth by providing population and employment forecasts for upper- and single-tier municipalities within the Greater Golden Horseshoe.

On August 28, 2020, Amendment 1 to A Place to Grow and Proposed Lands Needs Assessment Methodology came into force and effect. The upper- and single-tier municipalities were then required to conduct a Municipal Comprehensive Review (MCR) to ensure that their Official Plans conformed with provincial plans and policies.

### 2.1.3 Greenbelt Plan

The Greenbelt Plan consists of policies and schedules aimed to permanently protect the agricultural land base and the ecological, hydrological features, areas, and functions within the Greenbelt, which is in Ontario's Greater Golden Horseshoe region. More specifically, the Greenbelt Area includes lands within the Niagara Escarpment Plan Area, the Oak Ridges Moraine Area, the Parkway Belt West Area, and lands designated as Protected Countryside and as Urban River Valley. The vision for these areas is a broad band of permanently protected land that:

- Protects against the loss and fragmentation of the agricultural land base and supports agriculture as the predominant land use.
- Gives permanent protection to the natural heritage and water resource systems that sustain ecological and human health and that form the environmental framework around, which major urbanization in south-central Ontario will be organized.
- Provides for a diverse range of economic and social activities associated with rural communities, agriculture, tourism, recreation, and resource uses.
- Builds resilience to and mitigates climate change.

The south section of the Warden Avenue Study Area is within the Urban River Valley and Protected Countryside designations of the Greenbelt Plan. The north section of the Kennedy Road Study Area is within the Protected Countryside designations of the Greenbelt Plan. The Study Areas and Greenbelt Plan areas are shown in Figure 2.

Figure 2: Greenbelt Plan Areas


Greenbelt Plan - Designation
$\square$ Niagara Escarpment Plan
Oak Ridges Moraine Conservation Plan Protected Countryside Urban River Valley $\square$ Study Area Boundaries

### 2.1.4 Oak Ridges Moraine Conservation Plan

As part of the Greenbelt, the Oak Ridges Moraine is an environmentally sensitive, geological landform. The Oak Ridges Moraine Conservation Plan was established in 2002. It provides direction on land use and resource management for the land and water located within the moraine to protect ecological and hydrological features and functions. No section of the Warden Avenue and Kennedy Road Study Areas fall within the boundary of the Oak Ridges Moraine.

### 2.2 York Region Planning Context and Related Studies

### 2.2.1 York Region Official Plan

The Official Plan contains policies and schedules to accommodate future growth and development, while meeting the needs of existing residents and businesses in the Region. It provides directions and policies that guide economic, environmental, and community planning decisions. The 2019 consolidation of the 2010 Official Plan is currently being updated.

York Region is currently undertaking a Municipal Comprehensive Review process to update population and employment forecasts and allocations, land needs budget, and Regional Official Plan policies. The Region has until July 1, 2022, to complete their Municipal Comprehensive Review and Growth Plan conformity exercise, which includes allocating provincial growth forecasts of approximately 2,020,000 persons and 990,000 jobs to York Region by 2051. York Region is expecting to complete their Regional Official Plan adoption by Council in Q2 2022.

### 2.2.2 York Region Strategic Vision (Vision 2051), 2011

Vision 2051 is York Region's long-term strategy. This policy document describes the Region's ideal vision of the next 40 years and describes the action plan. One of the goals in Vision 2051 is for York Region to have a diverse urban form that provides a variety of interesting and exciting places to live, work, and play. This includes having communities that are people-first and designed for healthy, active living and social inclusion, and are the heart of business, arts and culture, community life and services. This goal is supported by the following actions relevant to the Warden Avenue and Kennedy Road corridors:

- Planning mixed-use pedestrian environments with attractive streets, high-quality urban design, and a distinct sense of place.
- Planning for safe, compact, complete, walkable, mixed-use communities, towns and villages that offer residents the opportunity to work and live active and healthy lives in the same community throughout their lives.
- Planning for an integrated urban network of communities, human services, jobs, transportation, and infrastructure systems that connect people to places, jobs, and services.
- Achieving a transit-oriented urban form.
- Achieving better connections between where people live, work, learn and play.

Another goal of Vision 2051 is to provide a seamless network for mobility that provides accessibility to all destinations using diverse transportation options for people in all communities, promotes active healthy living and safely and efficiently moves people and goods. This goal is supported by the following actions relevant to the Warden Avenue and Kennedy Road corridors:

- Ensuring a compact, mixed-use built form that minimizes the need for travel and reduces dependence on single occupant vehicles.
- Implementing and supporting transportation demand management initiatives that reduce automobile dependence by enhancing opportunities for residents and workers to walk, cycle, take transit and carpool.
- Providing convenient and reliable alternative modes of travel and prioritizing walking, cycling, public transit and carpooling.
- Implementing a comprehensive pedestrian system and programs that encourage walking, cycling and transit use.
- Facilitating an on and off-road cycling network that connects municipal cycling networks and trail systems and creates a Regional spine that will facilitate transportation by bicycle and support the use of public transit.


### 2.2.3 York Region's Sustainability Strategy

The Sustainability Strategy provides a long-term framework for making smart decisions about growth management and all municipal responsibilities that better integrate the economy, environment, and community. One of the goals of the strategy is to create self-sustaining and healthy communities that emphasize the human condition. This goal recognizes the importance of land-use and infrastructure planning, human services, and fiscal impacts. The relevant actions to the Warden Avenue and Kennedy Road corridors include:

- Ensure that all residents and employees, including new immigrants, the elderly, young people, and the disabled have barrier-free, accessible, and affordable transportation.
- Continue to provide rapid transit and public transit with connecting pedestrianfriendly access routes.
- Apply Transportation Demand Management to increase transit usage, carpooling and alternative transportation modes to improve access and mobility.


### 2.2.4 York Region Transportation Master Plan

York Region's Transportation Master Plan (TMP) is the Region's long-term vision for York Region's transportation network, encompassing strategy, policy, and infrastructure. The plan looks ahead 30 years and considers the Region's transportation infrastructure needs to support growth and the changing needs of travellers. The plan supports healthy communities and economic growth by planning for safe, reliable travel, and efficient movement of goods.

The TMP was last conducted in 2016 and satisfies Phases 1 and 2 of the MCEA process. At the time of writing this report, York Region's 2022 Transportation Master Plan was being conducted to support the planned growth of 2,020,000 people and 990,000 jobs by 2051.

The purpose of the 2022 Transportation Master Plan, currently underway, is to plan, build, operate, and maintain a connected transportation network for all travellers that is safe, reliable, future ready, sustainable and balances the needs of the unique communities.

The guiding principles include:

- Safety.
- Inclusive and equitable.
- Protect the environment.
- Affordable today and tomorrow.
- Balance the needs of communities and commuters.
- Future-ready.

The strategic objectives include:

- Make the best use of the roads, structures, and services we provide.
- Encourage all types of travel.
- Provide a resilient transportation network that is adaptable to the changing environment.
- Enhance partnerships.
- Collaborative sharing and learning through engagement and education.


### 2.2.5 York Region's Pedestrian and Cycling Master Plan (2007)

This master plan guides the development of a convenient and continuous regional-scale pedestrian and cycling network. This network consists of Regional and local roads to ensure that cycling and walking are safe and effective modes of transportation. This plan recognizes that walking and cycling are affordable and environmentally friendly modes of travel. A shift from motorized vehicles to active transportation can mitigate ozone depletion, ground-level air pollution, the greenhouse effect, traffic noise and urban sprawl. Economic benefits included reduced health care costs, higher productivity, and tourism.

Other goals of this plan include providing a safe environment for pedestrian and cyclists and enhancing the overall user experience through the implementation of policies and design standards. This plan provides a long-term pedestrian and cycling plan that was updated in York Region's 2016 Transportation Master Plan.

### 2.2.6 York Region Transit Business Plan (2021-2025)

The 2021 - 2025 Business Plan describes how York Region Transit (YRT) will address the impact of the COVID-19 pandemic and the Regin's transit needs over the next 5 years. This business plan outlines the Frequent Transit Network (FTN) plan, which identifies high-ridership corridors where additional service is warranted. Kennedy Road and Warden Avenue between Elgin Mills Road East and Major Mackenzie Drive East were identified as frequent transit network routes. Near to these Study Areas, Major Mackenzie Drive East was identified as a bus rapid transit route and Elgin Mills Road East was identified as a frequent transit network route.

Frequent transit network provides more direct service and improved transfers between routes by creating a connective grid network of Viva and base routes operating along key corridors in urban areas of the Region. The FTN supports travel between Regional Centres and along Regional Corridors and supports more direct inter-regional trips.

### 2.2.7 York Region's 2019 Designing Great Regional Streets

These guidelines aim to improve regional streets based on an examination of various needs and objectives within rights-of-way and road design standards. These guidelines also integrate road design and land use context by taking a context-sensitive approach that promotes the following outcomes:

- Flexibility to design for community context through the EA process.
- Consistency with facility applications in similar contexts.
- Best practices and sound, professional judgment.
- A 'made-in-York-Region' approach that considers full lifecycle costs.

These guidelines include six road typologies that reflects the Region's aspirations for the Regional road network:

- City Centre Street
- Avenue
- Main Street
- Connector
- Rural Road
- Rural Hamlet Road

Warden Avenue and Kennedy Road are planned to be "Connectors". Connectors prioritize goods and vehicle movement, while also supporting transit and active transportation. These roads are predominantly situated in residential or industrial areas, with small- to medium- scale built form that is typically set back from the street.

Design opportunities include generous green boulevards and multi-use paths.

### 2.3 City of Markham Planning Policies and Related Studies

### 2.3.1 City of Markham Official Plan

The City's Official Plan sets out land use policies that guide future development and manage growth. The 2014 Official Plan was approved on June 12, 2014 and amended through the LPAT Partial Approval Order Update issued on April 9, 2018.

The 2014 Official Plan contains policies on transportation, services, and utilities. These policies aim to build a transportation system within the City of Markham, which provides for the following:

- Selective road capacity enhancements.
- Increased and enhanced transit services.
- Transit-supportive development.
- Transportation demand management.
- Active transportation.


### 2.3.2 Future Urban Area Conceptual Master Plan

Markham's Official Plan has identified 1,300 hectares of lands, which includes 975 hectares of developable land, in North Markham to be the 'Future Urban Area'. Approximately 700 hectares of the developable lands are designated 'Future Neighbourhood Area', which will accommodate approximately 45,000 persons by full build-out. Approximately 275 hectares of developable lands north of Elgin Mills Road are designated as 'Future Employment Area', which will accommodate approximately 19,000 jobs at full build-out. The 4 residential blocks within the FUA are shown in Figure 3.

Transportation System Technical Report
June 6, 2022

Figure 3: Residential Blocks within the FUA
Source: Public Open House \#2 Display Boards, North Markham Future Urban Area Collector Roads Network Class Environmental Assessments (October 2019)


### 2.3.3 Markham Active Transportation Master Plan ('Ride \& Stride') (2021)

The 2021 Markham Active Transportation Master Plan (ATMP) outlines the policies, directions, steps, and projects that need to be undertaken to provide safe, comfortable, and well-connected active transportation network in Markham. The City identified strengthening the active transportation network as an important initiative to combat climate change and safety for road users. The City's "Getting to Zero: Markham's Municipal Energy Plan" assumes that as part of future scenarios 50\% of trips with a length between 1 and 5 km shift to cycling by 2040, and $50 \%$ of the potential walking trips that were less than 2 km were not supporting the travel of another passenger were shifted to walking by 2050.

The ATMP proposed an ultimate cycling network that includes existing paved shoulders on Warden Avenue and a "Proposed Regional Network" facility on Kennedy Road between Major Mackenzie Drive East and Elgin Mills Road East. It also includes proposed boulevard MUPs on Elgin Mills Road between Warden Avenue and Kennedy Road, and on the proposed collector roads within the Future Urban Areas that will be connected to the study corridors.

The proposed active transportation network is shown in Figure 4.

Figure 4：Proposed Active Transportation Network（City of Markham Active Transportation Master Plan）

Adapted from：Ride \＆Stride：Markham Active Transportation Master Plan，Executive Summary（2021）


Existing Cycling Network

| $=$ | Bike Lane |
| :--- | :--- |
| Cycle Track |  |
| Multi－Use Path or Trail |  |
| Paved Shoulder |  |
| Shared Roadway |  |
| Shared Roadway with Urban |  |
| Shoulder |  |
| Trail |  |

Proposed Cycling Network
－eeos Bike Lane
－．．．．Protected Bike Lane or Cycle Track
－－ローロ Off－road Multi－use Trail
$\pm=\infty=-\quad$ Boulevard Multi－use Path
－e．0．Paved Shoulder
$=-\infty=0$ Shared Roadway
－－－Desired Connection
－モーロー・ Proposed Regional Network

## 2．3．4 Future Urban Area Collector Road Network Class Environmental Assessments

The North Markham Future Urban Area Collector Road Network Class Environmental Assessments carry forward the Conceptual Master Planning（CMP）process for the proposed collector roads identified in the Community Structure Plan．To address Phases 3 and 4 of the MCEA process，an environmental study report（ESR）for each block was planned to be filed．At the time of writing this report，ESRs were filed for the Berczy Glen Block，Robinson Glen Block，and Victoria Glen Block．The Angus Glen

Transportation System Technical Report
June 6, 2022

Block ESR has been appealed and currently on hold due to Ontario Land Tribunal (OLT) hearings.

### 2.3.5 Elgin Mills Road Municipal Class Environmental Assessment

The Elgin Mills Road Municipal Class Environmental Assessment from Woodbine Avenue to McCowan Road was commenced in May 2020. The study area intersects the Study Areas for the Warden Avenue and Kennedy Road MCEAs. This study includes two study areas, which include Study Area A and Study Area B as shown in Figure 5.

Figure 5: Elgin Mills Road East Class EA Study Area
Source: Notice of Public Online Engagement \#2, Elgin Mills Road Municipal Class Environmental Assessment.


For Study Area A, the City completed Phase 1 and Phase 2 of the MCEA process and, at the time of writing this report, is carrying out the remaining Phase 3 and Phase 4. For Study Area B, the City relied on York Region's 2016 Transportation Master Plan that addressed Phases 1 and 2 of the MCEA process. The City is incorporating the findings related to this portion of Elgin Mills Road into this study and at the time of writing this report is proceeding to carry out the remaining Phases 3 and 4.

Transportation System Technical Report
June 6, 2022

### 3.0 Data Collection

Data for conducting traffic analysis were obtained from York Region. The Region also provided historical collision data for the safety assessment.

### 3.1 Turning Movement Counts

The turning movements counts that were provided by the Region for the traffic analysis are shown in Table 1. Turning movement counts are provided in Appendix A.

## Table 1: Turning Movement Counts Provided by York Region

| Location | Date of Count |
| :--- | :---: |
| Kennedy Road and Elgin Mills Road East | January 21, 2009 |
| Kennedy Road and Major Mackenzie Drive East | March 22, 2017 |
| Warden Avenue and Elgin Mills Road East | November 11, 2009 |
| Warden Avenue and Major Mackenzie Drive East | January 14, 2009 |

### 3.2 Supplemental Traffic Studies

Due to the dated regional turning movement count provided, traffic count data for the following studies were reviewed:

- The Master Environmental Servicing Plan (MESP) Transportation Study Update for Angus Glen Landowner Group and Berczy Glen Landowner Group, dated August 2, 2019, was prepared by Poulos \& Chung.
- Robinson Glen Traffic Impact Study Update for Robinson Glen Landowners Group, dated January 2020, was prepared by WSP.
- Transportation Study Final Report - Environmental Assessment Study for Elgin Mills Road East between West of Woodbine Avenue to East of McCowan Road, dated November 22, 2021, was prepared by IBI Group.

Details regarding weekday turning movement counts for the WSP and Poulos \& Chung studies are shown in Table 2. The IBI Group transportation study uses these 2 weekday TMCs for the existing conditions traffic analysis.

Table 2: Supplemental Traffic Data Details

| Location | Date of Count |
| :--- | :---: |
| Kennedy Road and Elgin Mills Road East | June 14, 2018 |
| Warden Avenue and Elgin Mills Road East | 2018 |

The traffic count data is provided in Appendix A.

### 3.3 Annual Average Daily Traffic

York Region provided historical annual average daily traffic (AADT) volumes along Kennedy Road between Elgin Mills Road East and Major Mackenzie Drive East, Warden Avenue between Elgin Mills Road East and Heritage Hill Drive, and Warden Avenue between Elgin Mills Road East and Major Mackenzie Drive East. Historical AADT data is provided in Appendix A.

### 3.4 Existing Signal Timing Plans

Signal timing plans were provided by the Region for the 4 signalized intersections within the Study Areas. Detailed signal timing plans are provided in Appendix B.

### 3.5 Speed Studies

Speed studies were provided by the Region for Kennedy Road and Warden Avenue. The $15-\mathrm{min}$ and $60-\mathrm{min}$ speed data and traffic volumes were collected for Kennedy Road on June 12, 2019. The 60-min speed data and traffic volumes were collected for Warden Avenue on June 18, 2019. Speed studies are provided in Appendix C.

### 3.6 Collision Data

Historical collision data was provided by the Region for Kennedy Road and Major Mackenzie Drive East. The collision data was from January 1, 2010, to April 30, 2021. A copy of the collision data is provided in Appendix D.

### 3.7 York Region Transportation Forecasting Model

The Region provided EMME modelling files for the 2016 base year, and 2031 and 2041 horizon years based on an updated York Region model that was originally developed in 2005. The modelling period is the AM peak period. These EMME modelling files include:

- Model run inputs.
- Model run macros.
- Model user manual.
- Emmebank with a 2016 network
- Emmebank with a 2031 network that is based on the Region's 2021 10-Year Capital Program and a 2041 network based on the Region's 2016 TMP recommendations.

Transportation System Technical Report
June 6, 2022

### 4.0 Existing Transportation Conditions

### 4.1 Transportation System Inventory

### 4.1.1 Road Network

Kennedy Road and Warden Avenue are north-south rural arterial roads that run continuously throughout the City of Markham. They intersect Elgin Mills Road East to the north, which runs east-west and under the jurisdiction of the City of Markham, and Major Mackenzie Drive East, which also runs east-west and under the jurisdiction of York Region. The road characteristics of these roads within or near the Study Areas are shown in Table 3.
Table 3: Road Characteristics within or near the Study Area

| Name <br> (Approximate Length) | Jurisdiction | Classification | Number <br> of <br> Through <br> Lanes | Posted <br> Speed <br> Limit |
| :--- | :---: | :---: | :---: | :---: |
| Kennedy Road (2.0 km) | York Region | Regional Arterial <br> Road | 2 | $80 \mathrm{~km} / \mathrm{hr}$ |
| Warden Avenue (2.1 <br> $\mathrm{km})$ | York Region | Regional Arterial <br> Road | 2 | $80 \mathrm{~km} / \mathrm{hr}$ |
| Major Mackenzie Drive <br> East (2.1 km) | York Region | Regional Arterial <br> Road | 4 | $70 \mathrm{~km} / \mathrm{hr}$ |
| Elgin Mills Road East <br> $(2.0 \mathrm{~km})$ | City of <br> Markham | Town Arterial <br> Road | 2 | $60 \mathrm{~km} / \mathrm{hr}$ |

The roadway configuration is shown in Figure 6.

Figure 6: Roadway Configuration


### 4.1.2 Transit Network

Within the Study Areas, York Region Transit (YRT) currently has no transit along Warden Avenue and Kennedy Road between Major Mackenzie Drive and Elgin Mills Road.

Along Warden Avenue, south of Major Mackenzie Drive East, Toronto Transit Commission (TTC) operates the 68B bus. This route has a terminal at the Angus Glen Community Centre at Major Mackenzie Drive East, between Warden Avenue and Kennedy Road.

Along Kennedy Road, south of Major Mackenzie Drive East, YRT operates Bus \#8. This route terminates northbound at Prospector's Drive and Major Mackenzie Drive.

Along Major Mackenzie Drive, YRT operates Bus \#25 that runs from Markham Stouffville Hospital at 9th Line to Mackenzie Richmond Hill Hospital near Yonge Street. This bus has stops along Major Mackenzie Drive including at Warden Avenue and Kennedy Road.

YRT also operates special routes including Bus \#18 and Bus \#402. The \#18 and \#402 buses run between Markham Stouffville Hospital and Angus Glen Community Centre. The \#18 bus runs during rush hours only and the \#402 bus runs on school days only. These bus routes are shown in Figure 7.

Transportation System Technical Report
June 6, 2022

Figure 7: TTC and YRT Bus Routes Near the Study Area


### 4.1.3 Active Transportation Network

There are continuous 1.0 m to 3.0 m paved shoulders along Warden Avenue and Kennedy Road between Elgin Mills Road East and Major Mackenzie Drive East. The paved shoulders continue north of Elgin Mills Road East along Warden Avenue and Kennedy Road. Along Kennedy Road, south of Major Mackenzie Drive East, there are no bike-dedicated facilities. Along Warden Avenue, south of Major Mackenzie Drive East, a multi-use path is provided.

### 4.1.4 Goods Movement

There are no truck restrictions along Kennedy Road and Warden Avenue. In York Region's Strategic Goods Movement Network, these roads are identified as secondary goods movement corridors. York Region's 2016 Transportation Master Plan identified secondary arterial goods movement corridors as Regional arterial roads that have fewer than 2,500 trucks per 8-hour period and fewer than 10\% medium and heavy trucks. Highway 404, which is in proximity of the Study Areas, is classified as a highway goods

Transportation System Technical Report
June 6, 2022
movement corridor. This classification is given to 400-series freeways with more than 3,000 trucks per day and more than $5 \%$ medium and heavy trucks.

### 4.1.5 Road Right-of-Way Characteristics

The road right-of-way for Kennedy Road is 33.2 m to 47.4 m . The road right-of-way for Warden Avenue is 31.2 m to 50.2 m .

### 4.1.6 Existing Accesses

York Region manages new and re-development accesses onto regional roads using access management practices to preserve transportation capacity of regional roads for all modes of transportation. Access management establishes the recommendations for appropriate location and design elements of the proposed public roads and private entrances onto regional roads.

There is one intersecting road along Warden Avenue, between Major Mackenzie Drive East and Elgin Mills Road East, called Heritage Hill Drive. There are ten properties that require ten driveway accesses onto Warden Avenue. These properties are shown in Figure 8.

There are no intersecting roads along Kennedy Road, between Major Mackenzie Drive East and Elgin Mills Road East. There are 12 properties that require 13 driveway accesses onto Kennedy Road. These properties are shown in Figure 9.

Angus Glen Golf Club is located at 10080 Kennedy Road and is the largest development along Kennedy Road.

Transportation System Technical Report June 6, 2022

Figure 8: Properties with Driveway Accesses Along Warden Avenue


Figure 9: Properties with Driveway Accesses Along Kennedy Road


### 4.2 Travel Demand

### 4.2.1 Travel Patterns

Transportation Tomorrow Survey (TTS), a household survey conducted in the Greater Golden Horseshoe Area, provided 2016 travel patterns and mode shares for three Traffic Analysis Zones (TAZs) adjacent to the study corridors as shown in Figure 10. These 3 TAZs were 2381, 2413, and 2416.

Transportation System Technical Report
June 6, 2022

Figure 10: Traffic Analysis Zones Adjacent to the Study Corridors


All-day trip productions to planning districts for the year 2016 for the 3 TAZs are shown in Table 4. Most trips are destined to within the City of Markham, City of Toronto, and the Town of Whitchurch-Stouffville.

Table 4: All-Day Trip Productions from Traffic Analysis Zones Adjacent to the Study Corridors*

|  | Trip Productions (\# of trips) |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\underset{N}{N}$ | $\begin{aligned} & \frac{\varepsilon}{\pi} \\ & \frac{\Sigma}{\Sigma} \\ & \frac{\Sigma}{\omega} \\ & \Sigma \end{aligned}$ | $\begin{aligned} & \text { O} \\ & \text { 을 } \\ & \text { • } \end{aligned}$ | $\begin{aligned} & \text { 즌 } \\ & \frac{1}{2} \end{aligned}$ |  |  |  |  | $\begin{aligned} & \text { O } \\ & \text { O } \\ & \text { D } \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \frac{त}{n} \\ & \frac{1}{7} \end{aligned}$ |  | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathbf{D}} \\ & \stackrel{y}{\square} \\ & \stackrel{\sim}{0} \\ & \underset{\sim}{\mathbf{0}} \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{0}{0} \end{aligned}$ |
| 2381 | 1263 | 114 | 23 | 0 | 164 | 17 | 7 | 0 | 70 | 77 | 15 | 25 | 1,775 |
| 2413 | 27 | 12 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 42 |
| 2416 | 109 | 0 | 0 | 38 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 147 |

* Burnside analysis of 2016 Transportation Tomorrow Survey data

All-day trip attractions for the year 2016 for the 3 TAZs are shown in Table 5. Most trips to the Study Areas originate from the City of Markham, City of Toronto, and Town of Whitchurch-Stouffville.

Transportation System Technical Report
June 6, 2022

Table 5: All-Day Trip Attractions to Traffic Analysis Zones Adjacent to the Study Corridors*

|  | Trip Attractions (\# of trips) |  |  |
| :---: | :---: | :---: | :---: |
|  | TAZ |  |  |
|  | $\mathbf{2 3 8 1}$ | $\mathbf{2 4 1 3}$ | $\mathbf{2 4 1 6}$ |
| Markham | 109 | 1266 | 27 |
| Toronto | 0 | 133 | 12 |
| Newmarket | 0 | 10 | 0 |
| Aurora | 0 | 23 | 0 |
| Richmond Hill | 38 | 0 | 0 |
| Whitchurch-Stouffville | 0 | 164 | 0 |
| Halton Hills | 0 | 7 | 0 |
| Bradford-West Gwillimbury | 0 | 25 | 0 |
| Scugog | 0 | 0 | 3 |
| Whitby | 0 | 70 | 0 |
| Clarington | 0 | 77 | 0 |
| Total | $\mathbf{1 4 7}$ | $\mathbf{1 , 7 7 5}$ | $\mathbf{4 2}$ |

* Burnside analysis of 2016 Transportation Tomorrow Survey data


### 4.2.2 Mode Share

The mode share for trip productions from the three TAZs adjacent to the study corridors are shown in Table 6. Most trips from the three TAZs are made by driving or as a passenger in a private vehicle.

Table 6: Mode Share for Trip Productions*

|  | Trip Productions (\# of trips) |  | Mode Share (\% of trips) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Driving | Passenger | Transit | Driving | Passenger | Transit |
| 2381 | 66 | 81 | 0 | $45 \%$ | $55 \%$ | $0 \%$ |
| 2413 | 1289 | 449 | 35 | $73 \%$ | $25 \%$ | $2 \%$ |
| 2416 | 42 | 0 | 0 | $100 \%$ | $0 \%$ | $0 \%$ |

* Burnside analysis of 2016 Transportation Tomorrow Survey data

The mode share for trip attractions to the three TAZs adjacent to the study corridors are shown in Table 7. Most trips to the three TAZs are made by driving or as a passenger in a private vehicle.

Table 7: Mode Share for Trip Attractions*

|  | Trip Attractions (\# of trips) |  |  | Mode Share (\% of trips) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Driving | Passenger | Transit | Driving | Passenger | Transit |
| Toronto | 145 | 0 | 0 | $100 \%$ | $0 \%$ | $0 \%$ |
| Scugog | 3 | 0 | 0 | $100 \%$ | $0 \%$ | $0 \%$ |
| Whitby | 70 | 0 | 0 | $100 \%$ | $0 \%$ | $0 \%$ |
| Clarington | 38 | 38 | 0 | $50 \%$ | $50 \%$ | $0 \%$ |
| Newmarket | 10 | 0 | 0 | $100 \%$ | $0 \%$ | $0 \%$ |
| Aurora | 11 | 11 | 0 | $50 \%$ | $50 \%$ | $0 \%$ |

Transportation System Technical Report
June 6, 2022

|  | Trip Attractions (\# of trips) |  |  | Mode Share (\% of trips) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Driving | Passenger | Transit | Driving | Passenger | Transit |
| Richmond <br> Hill | 38 | 0 | 0 | $100 \%$ | $0 \%$ | $0 \%$ |
| Whitchurch- <br> Stouffville | 146 | 18 | 0 | $89 \%$ | $11 \%$ | $0 \%$ |
| Markham | 923 | 431 | 48 | $66 \%$ | $31 \%$ | $3 \%$ |
| Halton Hills | 0 | 7 | 0 | $0 \%$ | $100 \%$ | $0 \%$ |
| Bradford- <br> West <br> Gwillimbury | 13 | 13 | 0 | $50 \%$ | $50 \%$ | $0 \%$ |

* Burnside analysis of 2016 Transportation Tomorrow Survey data


### 4.2.3 Temporal Distribution of Traffic Volumes

The speed studies provided temporal distribution of traffic volumes for Kennedy Road and Warden Avenue between Major Mackenzie Drive East and Elgin Mills Road East.

The temporal distribution for Kennedy Road for the northbound and southbound direction is illustrated in Figure 11. In the northbound direction, the AM peak hour is 8 AM with 415 vehicles. The PM peak hour is shown to be 4 PM with 542 vehicles. In the southbound direction, the AM peak hour is 7 AM with 808 vehicles. The PM peak hour is shown to be 5 PM with 377 vehicles. The southbound direction is the peak direction in the AM peak hour. The northbound direction is the peak direction in the PM peak hour.
Figure 11: Temporal Distribution of Traffic Volumes for Kennedy Road


Transportation System Technical Report
June 6, 2022

The temporal distribution for Warden Avenue for the northbound and southbound direction is illustrated in Figure 12. In the northbound direction, the AM peak hour is 8 AM with 465 vehicles. The PM peak hour is shown to be 4 PM with 1,189 vehicles. In the southbound direction, the AM peak hour is 8 AM with 1,147 vehicles. The PM peak hour is shown to be 5 PM with 539 vehicles. The southbound direction is the peak direction in the AM peak hour. The northbound direction is the peak direction in the PM peak hour.

Figure 12: Temporal Distribution of Traffic Volumes for Warden Avenue


### 4.2.4 Traffic Speed Distribution

Traffic volumes by speed along Kennedy Road are illustrated in Figure 13 The posted speed along Kennedy Road is $80 \mathrm{~km} / \mathrm{hr}$. Speed attributes, shown in Table 8, along this roadway segment show that most vehicles are travelling above the posted speed in the southbound direction.

Figure 13: Traffic Volumes by Speed along Kennedy Road

Transportation System Technical Report
June 6, 2022


Table 8: Speed Attributes along Kennedy Road

| Speed (km/hr) | NB | SB |
| :---: | :---: | :---: |
| Average | $76 \mathrm{~km} / \mathrm{hr}$ | $83 \mathrm{~km} / \mathrm{hr}$ |
| Median Speed | $77 \mathrm{~km} / \mathrm{hr}$ | $84 \mathrm{~km} / \mathrm{hr}$ |
| 85th Percentile Speed | $86 \mathrm{~km} / \mathrm{hr}$ | $94 \mathrm{~km} / \mathrm{hr}$ |

Traffic volumes by speed along Warden Avenue, between Major Mackenzie Drive East and Heritage Hill Drive are illustrated in Figure 14. The posted speed along this segment of Warden Avenue is $60 \mathrm{~km} / \mathrm{hr}$. Speed attributes, shown in Table 9, along this roadway segment show that most vehicles are travelling above the posted speed in the southbound segment. Estimates of the median and 85th percentile speeds suggest that most vehicles are travelling above the speed limit in the northbound direction.

Figure 14: Traffic Volumes by Speed along Warden Avenue


Table 9: Speed Attributes along Warden Avenue

| Speed (km/hr) | NB* $^{*}$ | SB |
| :---: | :---: | :---: |
| Average | Not available | $74 \mathrm{~km} / \mathrm{hr}$ |
| Median Speed | $61 \mathrm{~km} / \mathrm{hr}-65 \mathrm{~km} / \mathrm{hr}$ | $75 \mathrm{~km} / \mathrm{hr}$ |

Transportation System Technical Report
June 6, 2022

| Speed (km/hr) | NB $^{*}$ | SB |
| :---: | :---: | :---: |
| 85th Percentile Speed | $66 \mathrm{~km} / \mathrm{hr}-70 \mathrm{~km} / \mathrm{hr}$ | $82 \mathrm{~km} / \mathrm{hr}$ |

*Speed attributes were not provided in the data for the northbound direction unlike for the southbound direction. The northbound speed attributes were estimated based on aggregated data provided.

### 4.2.5 Pedestrian Activity

There is limited data detailing the pedestrian activity along the Warden Avenue and Kennedy Road corridors. The turning movement counts indicated that in 2017, there were five pedestrians who crossed the Major Mackenzie Drive East and Kennedy Road intersection during the AM peak hour and four pedestrian crossings during the PM peak hour. The 2009 counts indicated that there were no pedestrian crossings at the other three intersections during the AM or PM peak hours.

Because there are also no sidewalks along Kennedy Road and Warden Avenue between Major Mackenzie Drive East and Elgin Mills Road East, limited pedestrian activity is expected under existing conditions.

### 4.2.6 Existing Conditions Traffic Volumes

### 4.2.7 Elgin Mills Road East Intersections

After review of the Elgin Mills Road East traffic counts at Warden Avenue and Kennedy Road, which were conducted in 2009, Burnside concluded that these traffic counts are not suitable for this study. These 2009 traffic counts were normalized to the 2021 existing conditions year using growth rates and data from adjacent intersections. However, Burnside confirmed that normalizing the counts does not adequately capture the volumes associated with developments, which occurred between the 2009 count year and year 2021.

Burnside reviewed the IBI transportation study (details provided in Section 3.2) and adopted the approach that was undertaken to derive 2021 existing conditions, which was based on 2018 traffic counts conducted for the WSP and Poulos \& Chung traffic studies. This approach was adopted for the Elgin Mills Road East intersections at Warden Avenue and Kennedy Road.

### 4.2.8 Major Mackenzie Drive East Intersections

Historic AADTs, consideration of traffic volumes at adjacent intersections, and understanding of the land use context were used to normalize the 2009 historic turning movements counts at the Major Mackenzie Drive East intersections at Warden Avenue and Kennedy Road to represent 2021 existing conditions.

Compound annual growth rates were calculated for the Warden Avenue AADTs as shown in Table 10.

## Table 10: Warden Avenue - Historic Average Annual Daily Traffic

Transportation System Technical Report
June 6, 2022

| Year | Average Annual Daily Traffic |  |
| :---: | :---: | :---: |
|  | Between Major Mackenzie Drive <br> East and Heritage Hill Drive | Between Heritage Hill Drive <br> and Elgin Mills Road East |
| 2010 | 5,257 | 6,869 |
| 2014 | - | - |
| 2018 | - | 11,481 |
| 2019 | 6,014 | - |
| Compound Annual <br> Growth Rate | $2 \%$ | $7 \%$ |

Based on the CAGRs calculated using AADTs along Warden Avenue, a 4\% CAGR, which is an approximate average of the 2 growth rates, was applied to Warden Avenue turning movements to derive 2021 existing conditions turning movements.

Compound annual growth rates were calculated for the Kennedy Road AADTs as shown in Table 11.

Table 11: Kennedy Road - Historic Average Annual Daily Traffic

| Year | Average Annual Daily Traffic |
| :---: | :---: |
|  | Between Major Mackenzie Drive East and Elgin Mills Road East |
| 2010 | 6,913 |
| 2014 | 7,318 |
| 2018 | 6,972 |

Based on the CAGRs calculated using AADTs along Kennedy Road, there was very limited traffic growth between 2010, which was the year of the traffic count, and the 2021 existing conditions year. Therefore, a $0.5 \%$ CAGR was applied to Kennedy Road turning movements to derive 2021 existing conditions turning movements.

No data was provided for Major Mackenzie Drive East. A 2\% CAGR was applied to Major Mackenzie Drive East turning movements, which is the typical traffic growth rate for a major arterial.

These traffic growth rate assumptions were validated by comparing the derived upstream and downstream traffic volumes between adjacent intersections. Burnside also compared the derived 2021 existing conditions traffic volumes to the 2014 balanced turning movement counts from the North Markham Future Urban Area Conceptual Master Plan (see Appendix A) to ensure the assumptions were reasonable. After the validation, Burnside believed that these growth rates were suitable for the traffic analysis.

### 4.2.9 Traffic Volumes

The resulting 2021 existing conditions traffic volumes after applying the traffic compound annual growth rates are shown in Figure 15.

Figure 15: Existing Balanced Traffic Volumes (2021)


### 4.3 Automobile Level of Service Evaluation

York Region's automobile level-of-service (LOS) and volume-to-capacity (v/c) target is LOS D or v/c of 0.85 or better for urban areas and LOS C or v/c of 0.70 for rural areas based on the Region's Mobility Plan Guidelines. The traffic analysis below analyzes the automobile performance within the Study Areas and evaluates the performance based on the targets.

### 4.3.1 Existing Link Capacity Analysis

To assess the transportation corridor requirements for number of lanes, auto link traffic volumes were reviewed. A link capacity analysis involves evaluating the directional traffic volumes within a midblock zone between intersections. Auto traffic volumes were used in this analysis, as shown in Figure 15 and trucks were not considered. An assessment is made only between auto traffic volumes and roadway capacity.

A link capacity analysis was conducted using normalized 2021 turning movement counts for Warden Avenue and Kennedy Road for the AM and PM peak hours as presented in Table 12to Table 15. Note that the Region's EMME model set the lane capacity for Warden Avenue at 900 vehicles/hour/lane (v/h/l) and Kennedy Road at 1,000 vehicles/hour/lane (v/h/l). This link capacity analysis used $900 \mathrm{v} / \mathrm{h} / \mathrm{l}$ for Warden Avenue and Kennedy Road, which is typically used for an arterial roadway considering

Transportation System Technical Report
June 6, 2022
only autos. Congested links are assumed to have an auto link volume-to-capacity ratios greater than 0.90 and are highlighted in the tables below in red.
Table 12: Link Capacity Analysis for Warden Avenue - AM Existing Conditions

| Warden Avenue | \# of Lanes | Total Capacity | Volume | v/c <br> ratio | \# of Lanes | Total Capacity | Volume | v/c ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Southbound |  |  |  | Northbound |  |  |  |
| South of Elgin Mills Road East | 1 | 900 | 1102 | 1.22 | 1 | 900 | 239 | 0.27 |
| North of Major Mackenzie Drive East | 1 | 900 | 1112 | 1.24 | 1 | 900 | 166 | 0.18 |

Table 13: Link Capacity Analysis of Warden Avenue - PM Existing Conditions

| Warden <br> Avenue | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Southbound |  |  |  |  |  |  |  |
| South of <br> Elgin Mills <br> Road East | 1 | 900 | 251 | 0.28 | 1 | 900 | 1037 | 1.15 |
| North of <br> Major <br> Mackenzie <br> Drive East | 1 | 900 | 222 | 0.25 | 1 | 900 | 1029 | 1.14 |

Table 14: Link Capacity Analysis for Kennedy Road - AM Existing Conditions

| Kennedy <br> Road | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 900 | 700 | 0.78 | 1 | 900 | 352 | 0.39 |
| North of <br> Major <br> Mackenzie <br> Drive East | 1 | 900 | 674 | 0.75 | 1 | 900 | 409 | 0.45 |

Table 15: Link Capacity Analysis of Kennedy Road - PM Existing Conditions

| Kennedy <br> Road | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Southbound |  |  |  |  |  |  |  |
| South of <br> Elgin Mills <br> Road East | 1 | 900 | 361 | 0.40 | 1 | 900 | 520 | 0.58 |
| North of <br> Major <br> Mackenzie <br> Drive East | 1 | 900 | 351 | 0.39 | 1 | 900 | 392 | 0.44 |

The link capacity analysis indicated that Warden Avenue is operating above total link capacity during both AM and PM peak hours. The maximum link volume-to-capacity ratio is 1.24 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 1.15 in the northbound direction during the PM peak hour.
This link capacity analysis indicated that Kennedy Road is operating below total link capacity during both AM and PM peak hours The maximum link volume-to-capacity ratio is 0.78 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 0.58 in the northbound direction during the PM peak hour.

### 4.3.2 Existing Traffic Operations

Intersection operations were assessed for intersections in the Study Areas using the software program Synchro 11, which employs methodology from the Highway Capacity Manual (HCM 2000 and HCM 2010), published by the Transportation Research Board National Research Council. Synchro 11 can analyze both signalized and unsignalized intersections in a road corridor or network considering the spacing, interaction, queues and operations between intersections. The analysis contained within this report utilize the HCM2000 techniques / methodology within the Synchro software package.

## Signalized Intersections

Signalized intersection analysis considers two separate measures of performance:

- The capacity of all intersection movements, which is based on a volume to capacity ratio that measure of the degree of capacity utilized.
- The level of service (LOS) for all intersection movements, which is based on the average control delay per vehicle for the various movements through the intersection and overall. Delay is an indicator of how long a vehicle must wait to complete a movement and is represented by a letter between $A$ and $F$, with $F$ being the longest delay. The link between LOS and delay (in seconds) for signalized intersections is summarized below.

Transportation System Technical Report
June 6, 2022

| Level of Service | Control Delay per Vehicle (seconds) |
| :---: | :---: |
| A | $\leq 10$ |
| B | $>10-20$ |
| C | $>20-35$ |
| D | $>35-55$ |
| E | $>55-80$ |
| F | $>80$ |

It is not possible to have a v/c ratio greater than 1.0 for existing conditions, since the existing demand is being served by the existing timing plan; therefore, the software is underestimating the capacity of these movements. This could be due to several reasons including higher saturation flows, more aggressive drivers, vehicles utilizing more of the yellow or all red phases, etc. Lost time adjustments were applied to six critical movements in the AM peak hour and two critical movements in the PM peak hour with $\mathrm{v} / \mathrm{c}$ ratios greater than 1 to reflect drivers utilizing the yellow and all red phases. These critical movements in the AM peak hour include:

- 2.5 seconds for the southbound through movement at Warden Avenue and Elgin Mills Road East.
- 2.5 seconds for the northbound left movement at Kennedy Road and Elgin Mills Road East.
- 2.5 seconds for the westbound through movement at Kennedy Road and Major Mackenzie Drive East.
- 2.0 seconds for the northbound left movement at Kennedy Road and Major Mackenzie Drive East.
- 2.5 seconds for the westbound through movement at Warden Avenue and Major Mackenzie Drive East.
- 2.5 seconds for the southbound through movement at Warden Avenue and Major Mackenzie Drive East.

These critical movements in the PM peak hour include:

- 2.5 seconds for the northbound through movement at Warden Avenue and Elgin Mills Road East.
- 2.5 seconds for the eastbound through movement at Warden Avenue and Major Mackenzie Drive East.

Loss time adjustments were included in calibrated AM and PM peak hour Synchro scenarios. Baseline and calibrated Synchro results are provided in Appendix E. Calibrated results are reported in the subsequent sections.

The existing intersection operations were analyzed for the weekday AM and weekday PM peak hours based on the road network illustrated in Figure 6 and the existing balanced peak hour traffic volumes provided Figure 15. The signalized intersection operation results for Warden Avenue and Kennedy Road are provided in Table 16. A summary of all intersection operations is shown in Figure 16. Critical movements are highlighted in red in Table 18. These movements are categorized as critical when the

Transportation System Technical Report
June 6, 2022

LOS is E or F or if the volume-to-capacity (v/c) ratio is 0.85 or higher as per York Region's Transportation Mobility Plan Guidelines.

Table 16: Existing Conditions Traffic Operations Analysis

| Movement | Weekday AM Peak Hour |  | Weekday PM Peak Hour |  |
| :---: | :---: | :---: | :---: | :---: |
|  | v/c | LOS | v/c | LOS |
| Warden Avenue and Elgin Mills Road East |  |  |  |  |
| Overall | 1.19 | E | 1.05 | D |
| EBL | 0.21 | B | 0.09 | B |
| EBTR | 0.66 | B | 0.94 | D |
| WBL | 0.60 | B | 0.17 | B |
| WBT | 1.03 | E | 0.35 | B |
| NBL | 0.82 | E | 0.37 | B |
| NBT | 0.25 | B | 1.07 | E |
| NBR | 0.01 | B | 0.15 | B |
| SBL | 0.02 | B | 0.11 | B |
| SBT | 1.25 | F | 0.24 | B |
| SBR | 0.09 | B | 0.01 | B |
| Warden Avenue and Major Mackenzie Drive East |  |  |  |  |
| Overall | 1.38 | F | 1.03 | E |
| EBL | 0.02 | C | 0.11 | C |
| EBT | 0.66 | D | 1.15 | F |
| EBR | 0.09 | C | 0.10 | C |
| WBL | 0.87 | D | 0.66 | D |
| WBT | 1.12 | F | 0.37 | B |
| WBR | 0.01 | B | 0.02 | B |
| NBL | 0.48 | C | 0.57 | C |
| NBT | 0.11 | C | 0.83 | D |
| NBR | 0.05 | C | 0.45 | C |
| SBL | 0.08 | C | 0.48 | D |
| SBT | 1.69 | F | 0.41 | D |
| SBR | 0.06 | C | 0.01 | D |
| Kennedy Road and Elgin Mills Road East |  |  |  |  |
| Overall | 1.08 | D | 0.65 | B |
| EBL | 0.11 | B | 0.11 | B |
| EBT | 0.28 | B | 0.82 | C |
| EBR | 0.06 | B | 0.07 | B |
| WBL | 0.18 | B | 0.10 | B |
| WBT | 0.94 | D | 0.28 | B |
| WBR | 0.01 | B | 0.00 | B |
| NBL | 1.11 | F | 0.19 | B |
| NBT | 0.35 | B | 0.51 | B |
| NBR | 0.00 | B | 0.03 | B |
| SBL | 0.04 | B | 0.03 | B |

Transportation System Technical Report
June 6, 2022

| Movement | Weekday AM Peak Hour |  | Weekday PM Peak Hour |  |
| ---: | :---: | :---: | :---: | :---: |
|  | v/c | LOS | v/c | LOS |
| SBT | 0.93 | D | 0.32 | B |
| SBR | 0.08 | B | 0.01 | B |
| Kennedy Road and Major Mackenzie Drive East |  |  |  |  |
| Overall | 1.00 | E | 0.92 | C |
| EBL | 0.25 | C | 0.10 | B |
| EBT | 0.39 | C | 0.97 | D |
| EBR | 0.16 | C | 0.21 | B |
| WBL | 0.43 | B | 0.52 | C |
| WBT | 1.09 | F | 0.40 | B |
| WBR | 0.10 | B | 0.02 | B |
| NBL | 0.98 | F | 0.79 | D |
| NBT | 0.24 | C | 0.38 | D |
| NBR | 0.06 | C | 0.16 | C |
| SBL | 0.35 | D | 0.59 | D |
| SBT | 0.75 | D | 0.52 | D |
| SBR | 0.08 | D | 0.02 | D |

Figure 16: Summary of Existing Conditions Traffic Operations Analysis


Transportation System Technical Report
June 6, 2022

### 4.3.3 Existing Intersection Queues

The 95th percentile queues for each movement at the study intersections were extracted from the Synchro 11 analysis and compared to the available storage length as shown in Table 17. This analysis showed that queues for all movements at all intersections during both the AM and PM peak hours can be accommodated within the available storage. The detailed queuing results are provided in Appendix E.

Transportation System Technical Report
June 6, 2022

Table 17: Existing Conditions Queue Summary

| Movement | Existing Storage/ Link Distance (m) | Weekday AM Peak Hour | Weekday PM Peak Hour |
| :---: | :---: | :---: | :---: |
|  |  | 95th \% Queue (m) | 95th \% Queue (m) |
| Warden Avenue and Elgin Mills Road East |  |  |  |
| EBL | 70 | 7 | 9 |
| EBTR | 400+ | 70 | 145 |
| WBL | 80 | 33 | 7 |
| WBTR | 400+ | 161 | 37 |
| NBL | 125 | 33 | 31 |
| NBT | 500+ | 26 | 183 |
| NBR | 30 | 0 | 16 |
| SBL | 100 | 3 | 5 |
| SBT | 500+ | 204 | 27 |
| SBR | 30 | 10 | 1 |
| Warden Avenue and Major Mackenzie Drive East |  |  |  |
| EBL | 55 | 2 | 13 |
| EBT | 500+ | 102 | 357 |
| EBR | 85 | 16 | 16 |
| WBL | 145 | 97 | 37 |
| WBT | 450+ | 352 | 72 |
| WBR | 100 | 0 | 1 |
| NBL | 60 | 19 | 58 |
| NBT | 500+ | 20 | 152 |
| NBR | 60 | 8 | 68 |
| SBL | 140 | 14 | 19 |
| SBT | 550+ | 483 | 63 |
| SBR | 80 | 10 | 0 |
| Kennedy Road and Elgin Mills Road East |  |  |  |
| EBL | 120 | 5 | 10 |
| EBT | 350+ | 29 | 97 |
| EBR | 50 | 8 | 8 |
| WBL | 150 | 14 | 5 |
| WBT | 400+ | 133 | 29 |
| WBR | 60 | 0 | 0 |
| NBL | 125 | 48 | 17 |
| NBT | 300+ | 38 | 60 |
| NBR | 45 | 0 | 4 |
| SBL | 40 | 5 | 4 |
| SBT | 300+ | 140 | 37 |
| SBR | 35 | 9 | 0 |
| Kennedy Road and Major Mackenzie Drive East |  |  |  |
| EBL | 55 | 8 | 8 |
| EBT | 500+ | 74 | 300 |

Transportation System Technical Report
June 6, 2022

| Movement | Existing <br> Storage/ Link <br> Distance (m) | Weekday AM Peak <br> Hour | Weekday PM Peak <br> Hour |
| ---: | :---: | :---: | :---: |
|  | 95th \% Queue (m) | 95th \% Queue (m) |  |
| EBR | 60 | 16 | 31 |
| WBL | 250 | 36 | 19 |
| WBT | 360 | 382 | 77 |
| WBR | 150 | 15 | 0 |
| NBL | 120 | 100 | 63 |
| NBT | $300+$ | 36 | 45 |
| NBR | 60 | 12 | 21 |
| SBL | 195 | 31 | 34 |
| SBT | $550+$ | 83 | 40 |
| SBR | 60 | 16 | 0 |

### 4.4 Pedestrian Level of Service Evaluation

York Region's pedestrian level-of-service (LOS) target is LOS C or better. The analysis shown in Table 18 analyzes the pedestrian performance based on criteria from York Region's Mobility Plan Guidelines within the Study Areas and evaluates the performance based on the target. LOS that does not meet the target is highlighted in red.

Table 18: Pedestrian Level-of-Service Evaluation

| Intersection | Direction | Segment |  | Intersection LOS |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Description | LOS |  |
| Elgin Mills Road East and Warden Avenue | Eastbound | Elgin Mills Road East | E | C |
|  | Westbound | Elgin Mills Road East | E | C |
|  | Northbound | Warden Avenue | E | E |
|  | Southbound | Warden Avenue | E | C |
| Elgin Mills Road East and Kennedy Road | Eastbound | Elgin Mills Road East | E | C |
|  | Westbound | Elgin Mills Road East | E | C |
|  | Northbound | Kennedy Road | E | C |
|  | Southbound | Kennedy Road | E | E |
| Major Mackenzie Drive East and Warden Avenue | Eastbound | Major Mackenzie Drive East | C | C |
|  | Westbound | Major Mackenzie Drive East | B | B |
|  | Northbound | Warden Avenue | E | C |
|  | Southbound | Warden Avenue | C | C |
| Major Mackenzie Drive East and Kennedy Road | Eastbound | Major Mackenzie Drive East | B | B |
|  | Westbound | Major Mackenzie Drive East | C | C |
|  | Northbound | Kennedy Road | E | C |

Transportation System Technical Report
June 6, 2022

| Intersection | Direction | Segment |  | Intersection |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Description | LOS | LOS |

Improvements to the pedestrian environment should be implemented at the intersections of Elgin Mills Road East at Warden Avenue and Kennedy Road. Improvements to the pedestrian environment should be implemented on Kennedy Road and Warden Avenue between Elgin Mills Road East and Major Mackenzie Drive East.

Because the area is being redeveloped and reconstructed, opportunities to improve the pedestrian environment should be explored at the Major Mackenzie Drive East intersections at Warden Avenue and Kennedy Road concurrently with the improvements to the rest of the study corridor to ensure consistency and connectivity.

Transportation System Technical Report
June 6, 2022

### 4.5 Cycling Level of Service Evaluation

York Region's cycling level-of-service (LOS) target is LOS C or better. The analysis shown in Table 19 analyzes the cycling performance based on criteria from York Region's Mobility Plan Guidelines. LOS that does not meet the target is highlighted in red.

Table 19: Cycling Level-of-Service Evaluation

| Intersection | Direction | Segment |  | Intersection LOS |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Description | LOS |  |
| Elgin Mills Road East and Warden Avenue | Eastbound | Elgin Mills Road East | E | E |
|  | Westbound | Elgin Mills Road East | E | E |
|  | Northbound | Warden Avenue | E | E |
|  | Southbound | Warden Avenue | E | E |
| Elgin Mills Road East and Kennedy Road | Eastbound | Elgin Mills Road East | E | E |
|  | Westbound | Elgin Mills Road East | E | E |
|  | Northbound | Kennedy Road | E | E |
|  | Southbound | Kennedy Road | E | E |
| Major Mackenzie Drive East and Warden Avenue | Eastbound | Major Mackenzie Drive East | C | C |
|  | Westbound | Major Mackenzie Drive East | C | C |
|  | Northbound | Warden Avenue | E | F |
|  | Southbound | Warden Avenue | C | C |
| Major Mackenzie Drive East and Kennedy Road | Eastbound | Major Mackenzie Drive East | C | C |
|  | Westbound | Major Mackenzie Drive East | C | C |
|  | Northbound | Kennedy Road | D | E |
|  | Southbound | Kennedy Road | C | C |

Improvements to the cycling environment should be implemented at the intersections of Elgin Mills Road East at Warden Avenue and Kennedy Road. Improvements to the cycling environment should be implemented on Kennedy Road and Warden Avenue between Elgin Mills Road East and Major Mackenzie Drive East.

Because the area is being redeveloped and reconstructed, opportunities to improve the cycling improvement should be explored at the Major Mackenzie Drive East intersections at Warden Avenue and Kennedy Road concurrently with the improvements to the rest of the study corridor to ensure consistency and connectivity.

### 4.6 Transit Level of Service Evaluation

There is no transit service currently provided within the Study Areas. The Region supports transit as a robust transit network helps support growth to key centres and corridors and minimizes the need for travel and reduces dependence on single occupant vehicles. This sustainable mode of transportation also helps mitigate climate change, which is another important objective for York Region.

Future transit service should consider the following three criteria outlined in York Region's Mobility Plan Guidelines:

- Access to transit stops.
- Transit headways.
- Transit vehicle performance at the intersection approach.

York Region's transit level-of-service (LOS) target is LOS C or better for Access to Transit Stops and Transit Headways. The target is LOS D for Intersection Approach.

### 4.7 Traffic Safety

This section contains a safety assessment completed to identify and mitigate potential safety related concerns. The safety assessment consists of an in-office review of historical collision data. The in-office review helped identify any patterns with respect to collision type, direction, severity, and other contributing factors. These collision analysis findings provide an understanding of the overall safety performance of the study corridor, for intersections and midblock segments.

### 4.7.1 Review of Collision Data

York Region provided historical collision records from January 1, 2010, to April 30, 2021, along the two study corridors. Data for collisions during year 2020 and 2021 were removed to ensure that full years and years that were not affected by travel restrictions due to the COVID-19 pandemic were assessed. Five years of collision records between year 2015 to 2019 were analyzed to represent existing conditions. During this time period there were 88 collisions along and at the intersections of Warden Avenue and 72 collisions along and at the intersections of Kennedy Road.

### 4.7.2 Collision Rates

Collision rates were calculated for intersection and roadway segments using the following formulas:
Segment Collision Rate $=\frac{\text { Number of collisions x 1,000,000 }}{\text { Average Annual Daily Traffic x } 365 \times \text { Length of Segment } \times \text { Years }}$
Intersection Collision Rate $=\frac{\text { Number of collisions x 1,000,000 }}{\text { Average Annual Daily Traffic x } 365 \times \text { Years }}$

Transportation System Technical Report
June 6, 2022

Annual Average Daily Traffic (AADT) was estimated by multiplying the sum of the AM and PM peak hour volumes by 5 , and then adding together the total entering traffic volume. A hotspot, a location where safety countermeasures should be considered, is identified if the collision rate exceeds 1 or 1.5 crashes per million kilometres traveled.

A summary of the segment and intersection collision rates are illustrated in Figure 17. Warden Avenue, between Heritage Hill Drive and Major Mackenzie Drive East, is a possible hotspot. Its collision rate is highlighted in red.

Figure 17: Summary of Intersection and Segment Collision Rates


Details for the segment collision rates are shown in Table 20. Details for the intersection collision rates are shown in Table 21. Collision rates higher than 1.5 are highlighted in red.

Transportation System Technical Report
June 6, 2022

Table 20: Segment Collision Rates (2015-2019)

| Location | AADT <br> (year) | Number of <br> Collisions <br> (2015 to <br> 2019) | Segment <br> Length (km) | Segment <br> Collision <br> Rate |
| :--- | :---: | :---: | :---: | :---: |
| Warden Avenue between <br> Major Mackenzie Drive <br> East and Heritage Hill <br> Drive | 11,481 <br> $(2018)$ | 8 | 0.265 | 1.80 |
| Warden Avenue between <br> Heritage Hill Drive and <br> Elgin Mills Road East | 6,014 <br> $(2019)$ | 9 | 1.735 | 0.47 |
| Kennedy Road between <br> Major Mackenzie Drive <br> East and Elgin Mills Road <br> East | 6,972 <br> $(2017)$ | 10 | 2.000 | 0.65 |

Table 21: Intersection Collision Rates (2015-2019)

| Intersection | AADT <br> (estimated 2019) | Number of <br> Collisions | Intersection <br> Collision Rate |
| :--- | :---: | :---: | :---: |
| Warden Avenue <br> and Elgin Mills <br> Road East | 14,847 | 20 | 0.74 |
| Warden Avenue <br> and Major <br> Mackenzie Drive <br> East | 53,380 | 49 | 0.51 |
| Kennedy Road and <br> Elgin Mills Road <br> East | 11,046 | 15 | 0.74 |
| Kennedy Road and <br> Major Mackenzie <br> Drive East | 40,776 | 47 | 0.66 |

The collision rates for Warden Avenue and Kennedy Road suggest that no additional safety countermeasures are immediately required. Although the frequency of collisions between 2015 to 2019 at the Kennedy Road and Major Mackenzie intersection and the Warden Avenue and Major Mackenzie intersection are much higher than at the Elgin Mills Drive Road East intersections, the collision rates suggest that the Major Mackenzie Drive East intersections do not reflect collision trends requiring mitigation. The higher frequency of collisions at the Major Mackenzie Drive East intersections can be attributed to the higher traffic volumes entering the intersection.

The segment collision rate for Warden Avenue between Major Mackenzie Drive East and Heritage Hill Drive suggests that the segment may have potential for safety improvement. Detailed collision analysis can be found for this segment in Section 4.7.8.

Transportation System Technical Report
June 6, 2022

### 4.7.3 Collisions by Severity

The collisions by severity are illustrated within the Study Areas in Figure 18. Details of the collisions by severity for Warden Avenue are shown in Table 22. Details of the collisions by severity for Warden Avenue are shown in Table 23.

Property damage is the dominant type of collision whether it is a roadway segment or at the intersections. Property damage accounted for most collisions along and at the intersections of Warden Avenue and Kennedy Road. There were no fatal collisions along Warden Avenue and Kennedy Road between 2015 to 2019.

There were 7 injury-type collisions at the Elgin Mills Road East and Kennedy Road intersection accounting for $47 \%$ of the collisions at the intersection. Although there were a small number of collisions, and the severity of collisions may represent an opportunity for improvement, such as speed management.

Figure 18: Warden Avenue and Kennedy Road - Collisions by Severity (2015-2019)


Transportation System Technical Report
June 6, 2022

Table 22: Warden Avenue - Collisions by Severity (2015-2019)

|  | Property Damage Only \% (\# of collisions) | Non-Fatal Injury \% (\# of collisions) | $\begin{gathered} \text { Fatal Injury } \\ \% \\ \text { (\# of } \\ \text { collisions) } \end{gathered}$ | Total \% (\# of collisions) |
| :---: | :---: | :---: | :---: | :---: |
| At Elgin Mills Road East | $\begin{aligned} & \hline 90 \% \\ & (18) \end{aligned}$ | $\begin{gathered} 10 \% \\ (2) \end{gathered}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{gathered} 100 \% \\ (20) \end{gathered}$ |
| Between Heritage Hill Drive and Elgin Mills Road East | $\begin{gathered} 78 \% \\ (7) \end{gathered}$ | $\begin{gathered} 20 \% \\ (2) \end{gathered}$ | 0\% <br> (0) | $\begin{gathered} 100 \% \\ (9) \end{gathered}$ |
| At Heritage Hill Drive | $100 \%$ <br> (2) | $0 \%$ (0) | $\begin{aligned} & 0 \% \\ & (0) \\ & \hline \end{aligned}$ | $100 \%$ <br> (2) |
| Between Heritage Hill Drive and Major Mackenzie Drive East | 88\% <br> (7) | $\begin{gathered} 13 \% \\ (1) \end{gathered}$ | $0 \%$ <br> (0) | $\begin{gathered} 100 \% \\ (8) \end{gathered}$ |
| At Major Mackenzie Drive East | $\begin{aligned} & 63 \% \\ & (31) \end{aligned}$ | $\begin{aligned} & 36 \% \\ & (18) \end{aligned}$ | $0 \%$ <br> (0) | $\begin{gathered} 100 \% \\ (49) \end{gathered}$ |

Table 23: Kennedy Road - Collisions by Severity (2015-2019)

|  | Property Damage Only \% (\# of collisions) | $\begin{gathered} \text { Non-Fatal } \\ \text { Injury } \\ \% \\ \text { \# of } \\ \text { collisions) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Fatal Injury } \\ \text { \% } \\ \text { (\# of } \\ \text { collisions) } \end{gathered}$ | ```Total \% (\# of collisions)``` |
| :---: | :---: | :---: | :---: | :---: |
| At Elgin Mills Road East | 53\% <br> (8) | $47 \%$ <br> (7) | $\begin{aligned} & 0 \% \\ & (0) \\ & \hline \end{aligned}$ | $\begin{gathered} 100 \% \\ (15) \end{gathered}$ |
| Between Elgin Mills Road East and Major Mackenzie Drive East | $\begin{gathered} 60 \% \\ (6) \end{gathered}$ | $\begin{gathered} 40 \% \\ (4) \end{gathered}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{gathered} 100 \% \\ (10) \end{gathered}$ |
| At Major Mackenzie Drive East | $\begin{aligned} & 64 \% \\ & (30) \end{aligned}$ | $\begin{aligned} & 36 \% \\ & (17) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{gathered} 100 \% \\ (47) \end{gathered}$ |

### 4.7.4 Collisions by Initial Impact Type

The collisions by initial impact type are illustrated for the Study Areas in Figure 19. The percentages for the top three impact types are shown at each location. Details of the collisions by initial impact type on Warden Avenue are shown in Table 24. Details of the collisions by initial impact type on Kennedy Road are shown in Table 25. At

Transportation System Technical Report
June 6, 2022
intersections, rear end, angle, and single motor vehicle (SMV) collisions are the most predominant.

There were seven SMV collisions along Kennedy Road between Elgin Mills Road East and Major Mackenzie Drive East out of a total of ten, accounting for 70\% of the collisions along that segment between year 2015 to 2019. Four of the seven SMV collisions involved an animal collision. Safety countermeasures to mitigate animal collisions may be needed along this roadway segment. These can include:

- Enhanced illumination as Kennedy Road currently does not have streetlighting.
- Wildlife crossings.
- Wildlife warning signs.

Figure 19: Warden Avenue and Kennedy Road - Collisions by Initial Impact Type (2015-2019)


Transportation System Technical Report
June 6, 2022

Table 24: Warden Avenue - Collisions by Initial Impact Type (2015-2019)

| Initial Impact Type | $\begin{gathered} \text { Rear End } \\ \% \\ \text { (\# of } \\ \text { collisions) } \end{gathered}$ | Turning Movement \% (\# of collisions) | SMV Other $\%$ (\# of collisions) | $\begin{gathered} \text { Angle } \\ \text { \% } \\ \text { (\# of } \\ \text { collisions) } \end{gathered}$ | Side swipe \% (\# of collisions) | ```Approaching``` | $\begin{gathered} \text { Other } \\ \% \\ \text { (\# of } \\ \text { collisions) } \end{gathered}$ | Total \% <br> (\# of collisions) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| At Elgin Mills Road East | $25 \%$ <br> (5) | $15 \%$ <br> (3) | 25\% <br> (5) | $20 \%$ <br> (4) | 15\% <br> (3) | $\begin{aligned} & \hline 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & \hline 0 \% \\ & (0) \end{aligned}$ | $\begin{gathered} 100 \% \\ (20) \end{gathered}$ |
| Between <br> Heritage Hill <br> Drive and Elgin <br> Mills Road East | 50\% <br> (5) | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $20 \%$ <br> (2) | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $10 \%$ <br> (1) | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $10 \%$ <br> (1) | $100 \%$ <br> (9) |
| At Heritage Hill Drive | $50 \%$ <br> (1) | $\begin{aligned} & \hline 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & \hline 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & \hline 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & \hline 0 \% \\ & (0) \end{aligned}$ | $50 \%$ <br> (1) | $\begin{aligned} & 0 \% \\ & \text { (0) } \end{aligned}$ | $100 \%$ <br> (2) |
| Between <br> Heritage Hill <br> Drive and Major <br> Mackenzie <br> Drive East | $75 \%$ <br> (6) | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $25 \%$ <br> (2) | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | 100\% <br> (8) |
| At Major Mackenzie Drive East | $\begin{aligned} & 37 \% \\ & (18) \end{aligned}$ | $\begin{gathered} 37 \% \\ (18) \end{gathered}$ | $\begin{aligned} & 8 \% \\ & (4) \end{aligned}$ | $10 \%$ <br> (5) | $\begin{aligned} & 4 \% \\ & (2) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & 4 \% \\ & (2) \end{aligned}$ | $\begin{gathered} 100 \% \\ (49) \end{gathered}$ |

Transportation System Technical Report
June 6, 2022

Table 25: Kennedy Road - Collisions by Initial Impact Type (2015-2019)

| Initial Impact Type | $\begin{gathered} \text { Rear End } \\ \text { \% } \\ \text { (\# of } \\ \text { collisions) } \end{gathered}$ | Turning Movement \% (\# of collisions) | SMV Other $\%$ \#\# of collisions) | $\begin{gathered} \text { Angle } \\ \text { \% } \\ \text { (\# of } \\ \text { collisions) } \end{gathered}$ | Side swipe $\%$ (\# of collisions) | ```Approaching``` | Other \% <br> (\# of collisions) | ```Total \% (\# of collisions)``` |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| At Elgin Mills Road East | 33\% <br> (5) | $\begin{aligned} & 7 \% \\ & (1) \\ & \hline \end{aligned}$ | 13\% <br> (2) | $40 \%$ <br> (6) | $\begin{aligned} & 7 \% \\ & (1) \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 0 \% \\ & (0) \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 0 \% \\ & (0) \end{aligned}$ | $\begin{gathered} \hline 100 \% \\ (15) \end{gathered}$ |
| Between Elgin Mills Road East and Major Mackenzie Drive East | $10 \%$ <br> (1) | 10\% <br> (1) | $70 \%$ <br> (7) | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | 10\% <br> (1) | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{gathered} 100 \% \\ (10) \end{gathered}$ |
| At Major Mackenzie Drive East* | $\begin{aligned} & 38 \% \\ & (18) \end{aligned}$ | $\begin{gathered} 36 \% \\ (17) \end{gathered}$ | $\begin{gathered} 2 \% \\ (1) \end{gathered}$ | $\begin{aligned} & 21 \% \\ & (10) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{aligned} & 0 \% \\ & (0) \end{aligned}$ | $\begin{gathered} 100 \% \\ (46) \end{gathered}$ |

*1 collision record recorded no initial impact type

Transportation System Technical Report
June 6, 2022

### 4.7.5 Collisions by Vulnerable Road Users

York Region identifies pedestrians and cyclists as the most vulnerable travellers on the road network. The rates of injuries they sustain are higher compared to other modes of travel.

On Warden Avenue, 2 of the 88 collisions between year 2015 to 2019 involved cyclists. One cyclist collision involved a cyclist going too fast for the conditions and hitting an automobile in clear, dry conditions. In the second cyclist collision, the motor vehicle failed to yield right-of-way in clear, dry conditions.

On Kennedy Road, 2 of the 72 collisions between year 2015 to 2019 involved cyclists. One cyclist collision involved a cyclist who failed to yield for the right-of-way in clear, dry conditions. In the second cyclist collision, the cyclist lost control and hit a stopped motor vehicle.

There were no pedestrian collisions during this time period.
Although the number of cyclist and pedestrian collisions were low, the alternative design concepts should consider the need to protect for vulnerable road users since active transportation activity is expected to increase along this corridor. Cyclists and pedestrians are also vulnerable road users, and they are at more risk for serious injury or death involving a motor vehicle collision.

### 4.7.6 Collisions by Year, Month, and Day of the Week

The number of collisions by year and severity are shown in Table 26 for Warden Avenue. In this 5-year period, there was no relationship between year and number of collisions.

Table 26: Warden Avenue - Collisions by Severity and Year (2015-2019)

|  | $\mathbf{2 0 1 5}$ | $\mathbf{2 0 1 6}$ | $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 8}$ | $\mathbf{2 0 1 9}$ | Total | \% |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P.D Only | 12 | 17 | 11 | 11 | 14 | 65 | $74 \%$ |
| Non-Fatal <br> Injury | 6 | 3 | 4 | 6 | 4 | 23 | $26 \%$ |
| Fatal Injury | 0 | 0 | 0 | 0 | 0 | 0 | $0 \%$ |
| Total | 18 | 20 | 15 | 17 | 18 | 88 | $100 \%$ |
| $\%$ | $20 \%$ | $23 \%$ | $17 \%$ | $19 \%$ | $20 \%$ | $100 \%$ |  |

The number of collisions by month and severity are shown in Table 27 for Warden Avenue. The months with the highest number of collisions during this 5 -year period were June, July, November, and December.

Transportation System Technical Report
June 6, 2022
Table 27: Warden Avenue - Collisions by Severity and Month (2015-2019)

|  | $\stackrel{\text { ¢ }}{\sim}$ | 윤 | $\sum_{\Sigma}^{\frac{\hbar}{\pi}}$ | $\frac{亠}{4}$ | $\sum_{\Sigma}^{\text {I }}$ | $\xlongequal[\leftrightharpoons]{\leftrightharpoons}$ | $\bar{\jmath}$ | $\frac{0}{3}$ | ○ © | せ | ㄹ | - | $\begin{aligned} & \overline{\Pi ँ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | \%* |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P.D Only | 5 | 5 | 3 | 5 | 2 | 6 | 7 | 5 | 8 | 3 | 7 | 9 | 65 | 74 |
| Non-Fatal Injury | 3 | 0 | 0 | 2 | 2 | 5 | 0 | 3 | 1 | 1 | 3 | 3 | 23 | 26 |
| Fatal Injury | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Total | 8 | 5 | 3 | 7 | 4 | 11 | 7 | 8 | 9 | 4 | 10 | 12 | 88 | 100 |
| \%* | 9 | 6 | 3 | 8 | 5 | 13 | 8 | 9 | 10 | 5 | 11 | 14 | 100 |  |

*\% unit not shown
The number of collisions by day and severity are shown in Table 28 for Warden Avenue. Wednesday had the highest number of collisions over this time period.

Table 28: Warden Avenue - Collisions by Severity and Day of the Week (2015 2019)

|  | Sun. | Mon. | Tues. | Wed. | Thu. | Fri. | Sat. | Total | $\%$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P.D Only | 8 | 10 | 8 | 16 | 9 | 8 | 6 | 65 | $74 \%$ |
| Non-Fatal <br> Injury | 1 | 4 | 4 | 4 | 6 | 2 | 2 | 23 | $26 \%$ |
| Fatal Injury | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | $0 \%$ |
| Total | 9 | 14 | 12 | 20 | 15 | 10 | 8 | 88 | $100 \%$ |
| $\%$ | $10 \%$ | $16 \%$ | $14 \%$ | $23 \%$ | $17 \%$ | $11 \%$ | $9 \%$ | $100 \%$ |  |

The number of collisions by year and severity are shown in Table 29 for Kennedy Road. In this 5-year period, there was no relationship between year and number of collisions.

Table 29: Kennedy Road - Collisions by Severity and Year (2015-2019)

|  | $\mathbf{2 0 1 5}$ | $\mathbf{2 0 1 6}$ | $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 8}$ | $\mathbf{2 0 1 9}$ | Total | $\mathbf{\%}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P.D Only | 13 | 12 | 6 | 7 | 6 | 44 | $61 \%$ |
| Non-Fatal <br> Injury | 7 | 6 | 3 | 6 | 6 | 28 | $39 \%$ |
| Fatal Injury | 0 | 0 | 0 | 0 | 0 | 0 | $0 \%$ |
| Total | 20 | 18 | 9 | 13 | 12 | 72 | $100 \%$ |
| $\%$ | $28 \%$ | $25 \%$ | $13 \%$ | $18 \%$ | $17 \%$ | $100 \%$ |  |

The number of collisions by month and severity are shown in Table 30 for Kennedy Road. The months with the highest number of collisions during this 5 -year period were April, September, January, and December.

Transportation System Technical Report
June 6, 2022
Table 30: Kennedy Road - Collisions by Severity and Month (2015-2019)

|  | 亭 | 윤 | $\sum_{i}^{\frac{1}{\pi}}$ | $\frac{\vdots}{4}$ | ${\underset{\Sigma}{\pi}}_{\lambda}^{\pi}$ | $\xlongequal[\leftrightharpoons]{\leftrightharpoons}$ | $\bar{\jmath}$ | $\frac{\text { 울 }}{3}$ | ○ | U | Z | O. | $\stackrel{\overline{\boxed{O}}}{\stackrel{0}{\circ}}$ | \% |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \hline \text { P.D } \\ & \text { Only } \end{aligned}$ | 6 | 4 | 5 | 5 | 1 | 2 | 3 | 2 | 5 | 3 | 3 | 5 | 44 | 61\% |
| Non- <br> Fatal <br> Injury | 1 | 2 | 1 | 5 | 1 | 3 | 4 | 2 | 2 | 3 | 2 | 2 | 28 | 39\% |
| Fatal Injury | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% |
| Total | 7 | 6 | 6 | 10 | 2 | 5 | 7 | 4 | 7 | 6 | 5 | 7 | 72 | 100\% |
| \%* | 10 | 8 | 8 | 14 | 3 | 7 | 10 | 6 | 10 | 8 | 7 | 10 | 100 |  |

*\% unit not shown
The number of collisions by day and severity are shown in Table 31 for Kennedy Road. Wednesday had the higher number of collisions over this time period.

Table 31: Kennedy Road - Collisions by Severity and Day of the Week (2015 2019)

|  | Sun. | Mon. | Tues. | Wed. | Thu. | Fri. | Sat. | Total | $\%$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P.D Only | 4 | 4 | 5 | 9 | 11 | 5 | 6 | 44 | $59 \%$ |
| Non-Fatal <br> Injury | 5 | 5 | 7 | 4 | 1 | 3 | 3 | 28 | $39 \%$ |
| Fatal Injury | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | $3 \%$ |
| Total | 9 | 9 | 12 | 13 | 12 | 8 | 9 | 72 | $100 \%$ |
| $\%$ | $13 \%$ | $13 \%$ | $17 \%$ | $18 \%$ | $17 \%$ | $11 \%$ | $13 \%$ | $100 \%$ |  |

### 4.7.7 Collisions by Environmental Conditions

The distribution of collisions by environmental conditions for Warden Avenue are shown in Table 32. No pattern was observed related to environmental patterns.
Table 32: Warden Avenue - Collisions by Environmental Conditions (2015-2019)

| Environmental <br> Conditions | Collisions | $\%$ |
| :---: | :---: | :---: |
| Clear | 64 | $73 \%$ |
| Rain | 15 | $17 \%$ |
| Snow or Drifting Snow | 8 | $9 \%$ |
| Strong wind | 1 | $1 \%$ |
| Total | 88 | $100 \%$ |

Transportation System Technical Report
June 6, 2022
The distribution of collisions by environmental conditions for Kennedy Road are shown in Table 33. No pattern was observed related to environmental patterns.

Table 33: Kennedy Road - Collisions by Environmental Conditions (2015-2019)

| Environmental <br> Conditions | Collisions | $\%$ |
| :---: | :---: | :---: |
| Clear | 53 | $75 \%$ |
| Snow or Drifting Snow | 11 | $16 \%$ |
| Rain | 7 | $10 \%$ |
| Total | 72 | $100 \%$ |

### 4.7.8 Collision Analysis for Warden Avenue between Major Mackenzie Drive East and Heritage Hill Drive

There were eight collisions between year 2015 to 2019 along Warden Avenue between Major Mackenzie Drive East and Heritage Hill Drive. The impact type and severity of those collisions are shown in Table 34.

Table 34: Warden Avenue between Major Mackenzie Drive East and Heritage Hill Drive by Impact and Severity (2015-2019)

| Impact Type | Severity | \# of collisions |
| :---: | :---: | :---: |
| Rear End | Property damage only | 5 |
| Rear End | Non-fatal | 1 |
| SMV - Animal | Property damage only | 1 |
| SMV - Speed too fast | Property damage only | 1 |

Five or $63 \%$ of the total collisions that occurred along this segment within the 5 -year period were rear end collisions that resulted in property damage. Speed was not explicitly recorded to be a factor in these five collisions suggesting that they were caused by careless driving. Because this segment is short (265 metres) and close to the Warden Avenue and Major Mackenzie Drive East intersection, safety countermeasures leading to the intersection could be implemented along this segment. These countermeasures can include:

- Electronic radar speed signs at the mid-block between Major Mackenzie Drive East and Heritage Hill Drive.
- Traffic calming: enhanced pavement markings, narrow width of road.
- Streetscape elements that can contribute to lower speeds.

Transportation System Technical Report
June 6, 2022

### 5.0 Future Transportation Conditions

### 5.1 Future Land Use

The Study Areas are within the City of Markham's Future Urban Area, which is planned to accommodate approximately 38,000 persons and approximately 19,000 jobs by full build-out year which is assumed to be year 2031.

The Region's updated transportation model provided population and employment forecasts by TAZ to year 2041 near the Study Areas, which is represented by TAZ 2381, 2413, and 2416, as shown in Figure 20.

Figure 20: Traffic Analysis Zones near the Study Areas


The population forecasts to year 2041 are shown in Table 35.
Table 35: Population Forecasts near the Study Areas

| TAZ | $\mathbf{2 0 1 6}$ | $\mathbf{2 0 3 1}$ | $\mathbf{2 0 4 1}$ |
| :---: | :---: | :---: | :---: |
| 2381 | 74 | 8,990 | 9,015 |
| 2413 | 37 | 14,102 | 14,141 |
| 2416 | 74 | 10,603 | 16,143 |
| Total | $\mathbf{1 8 6}$ | $\mathbf{3 3 , 6 9 5}$ | $\mathbf{3 9 , 2 9 9}$ |

The employment forecasts to year 2041 are shown in Table 36.
Table 36: Employment Forecasts near the Study Areas

| TAZ | $\mathbf{2 0 1 6}$ | $\mathbf{2 0 3 1}$ | $\mathbf{2 0 4 1}$ |
| :---: | :---: | :---: | :---: |
| 2381 | 13 | 2,479 | 2,491 |
| 2413 | 606 | 3,134 | 3,649 |
| 2416 | 3 | 1,012 | 1,848 |
| Total | $\mathbf{6 2 1}$ | $\mathbf{6 , 6 2 6}$ | $\mathbf{7 , 9 8 8}$ |

Transportation System Technical Report
June 6, 2022

### 5.2 Planned Road Network

The 2016 Transportation Master Plan recommended to widen the Warden Avenue and Kennedy Road corridors within the Study Areas from two to four lanes and construct to urban arterial standard. The TMP also recommended dedicated cycling facilities along both corridors. These facilities are defined as facilities that provide specific space for cyclists. In urban areas, dedicated facilities typically include bike lanes, buffered bike lanes, or multi-use paths while paved shoulders provide dedicated space on rural roads.

The 2016 TMP also recommended to widen Elgin Mills Road East between Woodbine Avenue to Kennedy Road from two to four lanes between year 2027 to 2031 and to widen between Kennedy Road to Donald Cousens Parkway between year 2032 to 2041.

The City of Markham's 2021 Active Transportation Master Plan maintains the existing paved shoulder along Warden Avenue between Major Mackenzie Drive East and Elgin Mills Road East in the City's proposed ultimate cycling network. A paved shoulder would not be consistent with the Region's plans to reconstruct Warden Avenue to an urban arterial standard with a dedicated cycling facility.

The City's ATMP was used as input for the Environmental Assessments of the two Study Areas. These Class EA studies will consider the recommendations and through the various assessments and investigations undertaken throughout the studies, the ATMP recommendations will be incorporated where appropriate.

There is an existing multi-use path along Warden Avenue between 16th Avenue and Major Mackenzie Drive East on the east side. Given that the area is planned for redevelopment with an urban cross section for Warden Avenue, it would be appropriate to continue the multi-use path north of Major Mackenzie Drive East on the east side for connectivity of the active transportation network. As mentioned earlier, the Region also plans to have dedicated cycling facilities along Kennedy Road.
The City of Markham's Future Urban Area Conceptual Master Plan recommended a comprehensive collector road network consisting of roads and active transportation infrastructure. This network is shown in Figure 21. The proposed signal and intersection configuration is shown Figure 22. These intersection configurations were assessed in the Collector Road Environmental Assessments.

Transportation System Technical Report
June 6, 2022
Figure 21: Proposed Community Structure Plan (Conceptual Master Plan)
Adapted from: Future Urban Area Conceptual Master Plan Volume 2 (October 2018)


Figure 22: Proposed Signal and Intersection Configuration
Source: Future Urban Area Conceptual Master Plan Volume 2 (October 2018)


The 2019 consolidation of York Region's Official Plan 2010 identified the planned right-of-way for Kennedy Road and Warden Avenue to be up to 43 metres. At the time of writing this memorandum, a draft updated Region Official Plan indicates the planned right-of-way for Kennedy Road and Warden Avenue to be up to 41 metres and up to 43 metres at intersections.

Transportation System Technical Report
June 6, 2022

### 5.3 Planned Transit Service

Kennedy Road and Warden Avenue between Elgin Mills Road East and Major Mackenzie Drive East are planned to be frequent transit network routes. Frequent Transit Network provides more direct service and improved transfers between routes by creating a connective grid network of Viva and base routes operating along key corridors in urban areas of the Region. The FTN supports travel between Regional Centres and along Regional Corridors and supports more direct inter-regional trips.

### 5.42041 Future Automobile Demand

The 2041 AM and PM future automobile demands were used to assess traffic operations along the Warden Avenue and Kennedy Road study corridors. The methodology to derive the 2041 future automobile demand is documented in the following section. The general methodology is outlined below:

1. Refine the Region's transportation models.
2. Derive appropriate compound annual growth rates based on refined models and available data.
3. Apply compound annual growth rates to the 2021 existing volumes as shown in Figure 15.

The traffic growth rates were calculated using peak direction modelled volumes in the AM peak hour, which is the time period of the EMME model. These traffic growth rates were applied to both directions of the existing conditions AM and PM traffic volumes derived from the turning movement counts to forecast 2041 traffic volumes

### 5.4.1 Transportation Model Refinements

The following transportation model refinements were made to the network in EMME:

- For the 2031 horizon year model, Warden Avenue was re-coded to two lanes per direction from Elgin Mills Road East to Donald Cousens Parkway. This network edit is consistent with the 2016 TMP. Warden Avenue between Major Mackenzie Drive to Donald Cousens Parkway (Project ID 2073) is planned to be widened from two to four lanes between 2027 to 2031. Originally, the network was coded to one lane per direction from Elgin Mills Road East to Donald Cousens Parkway.
- For the 2031 and 2041 horizon years, Warden Avenue and Kennedy Road between Major Mackenzie Drive East and Elgin Mills Road East were re-coded to 60 km/hr. to be consistent with the Region's vision for the corridors. The auto assignment was re-assigned to the network.

The peak direction (southbound) modelled volumes before and after the model refinements are shown in Figure 23 for the 2031 horizon year and in Figure 25 for the

Transportation System Technical Report
June 6, 2022
2041 horizon year. Original and refined EMME modelled volume plots for the 2031 and 2041 horizon years can be found in Appendix F.

The EMME model was primarily used to derive appropriate compound annual growth rates along Warden Avenue and Kennedy Road. More detailed traffic operations analysis was conducted in the Synchro. Therefore, the EMME model network was reviewed and found to be sufficient in representing the collector road system in the Future Urban Area for this stage of the transportation analysis. Traffic operations assessment that includes future accesses and intersections as a result of the planned collector road system will be documented in Transportation Technical Report \#2 which assesses the traffic operations of the preferred preliminary design concept.

Figure 23: 2031 Modelled Volumes Before and After Refinements


Transportation System Technical Report
June 6, 2022
Figure 24: 2041 Modelled Volumes Before and After Refinements


### 5.4.2 Traffic Growth Rates

A traffic compound annual growth rate (CAGR) analysis was completed based on the models used for York Region's 2016 Transportation Master Plan (see Section 5.4.1).

Growth rates were calculated for the northbound/southbound directions and eastbound/westbound directions separately. This growth rate analysis assumed that the CAGR for the associated turning movements are half the through movements. Growth rates are also based on peak direction in the AM peak hour. The Region's model is an AM peak period models.

An ‘unconstrained' growth rate approach was adopted which calculates growth rates with planned roadway improvements in the future horizon years. Unconstrained growth captures the full effects of latent demand so that all trips that desire to travel along the study corridors are not constrained by roadway capacity.

### 5.4.3 2016 to 2031 Compound Annual Growth Rate Analysis.

## Northbound/Southbound Compound Annual Growth Rate Analysis

A screenline was identified mid-block on Warden Avenue and on Kennedy Road between Major Mackenzie Drive East and Elgin Mills Road.

Transportation System Technical Report
June 6, 2022
A 2016 to 2031 traffic CAGR was calculated from the 2016 and 2031 York Region models. The 2031 model was refined to reflect the change in posted speed limits for the study corridors as outlined in Section 5.4.1. The results are shown in Table 37.
Table 37: Modelled Volumes for the 2016 to 2031 NB/SB Traffic Growth Rate

| Year | Average Modelled <br> Volume along <br> Warden Avenue | Average Modelled <br> Volume along <br> Kennedy Road | Screenline <br> Volumes |
| :--- | :---: | :---: | :---: |
| 2016 Base Year | 881 | 783 | 1,664 |
| 2031 Horizon Year <br> with Posted Speed <br> Limit Reduction | 1,338 | 1,137 | 2,475 |
| $2016-2031$ with <br> Posted Speed Limit <br> Reduction Auto <br> Traffic CAGR | $3 \%$ | $3 \%$ | $3 \%$ |

As a result of this analysis, a 3.0\% CAGR was applied to 2021 north/south through movements to forecast 2031 traffic volumes for this study. A 1.5\% CAGR was applied to turning movements.

## Eastbound/Westbound Compound Annual Growth Rate Analysis

Compound annual growth rates for through movements associated with Major Mackenzie Drive East and Elgin Mills Road East were derived based on modelled volumes from York Region's transportation models. Major Mackenzie Drive East and Elgin Mills Road East were analyzed separately due to their road functions. Major Mackenzie Drive East is a major arterial with high volumes and already approaching vehicular capacity in the existing conditions. There are no planned widenings associated with this roadway constraining the amount of traffic volume growth that can occur along this segment. Elgin Mills Road East is a City of Markham road that is performing below vehicular capacity in the existing conditions. There is also a 2-lane to 4 -lane widening planned for this roadway.

Based on the Region's modelled traffic volumes between Warden Avenue and Kennedy Road, for through movements associated with Major Mackenzie Drive East, a 0.5\% compound annual growth rate was applied to the 2021 existing conditions year to forecast 2031 traffic volumes as shown in Table 38. A $0.25 \%$ compound annual growth rate was applied to the turning movements.

Based on the Region's modelled traffic volumes between Warden Avenue and Kennedy Road, for through movements associated with Elgin Mills Road East, a 7\% compound annual growth rate was applied between year 2021 to 2031. A $2.0 \%$ compound annual growth rate was applied to the turning movements.

Transportation System Technical Report
June 6, 2022
Table 38: Modelled Volumes for the 2016 to 2031 EB/WB Traffic Growth Rate

| Year | Average Modelled <br> Volume along Major <br> Mackenzie Drive East | Average Modelled <br> Volume along Elgin <br> Mills Road East |
| :--- | :---: | :---: |
| 2016 Base Year | 1,874 | 568 |
| 2031 Horizon Year | 1,879 | 1,513 |
| 2011 - 2031 with Reduction <br> Auto Traffic CAGR | $0 \%$ | $7 \%$ |

### 5.4.4 2031 to 2041 Compound Annual Growth Rate Analysis

## Northbound/Southbound Compound Annual Growth Rate Analysis

A 2031 to 2041 traffic CAGR was calculated from the 2031 and 2041 York Region transportation models. These models were refined to reflect the change in posted speed limits for the study corridors as outlined in Section 5.4.1. The results are shown in Table 39.

Table 39: Modelled Traffic Volumes for the 2031 to 2041 NB/SB Traffic Growth Rate

| Year | Average <br> Modelled <br> Volume along <br> Warden Avenue | Average <br> Modelled <br> Volume along <br> Kennedy Road | Screenline <br> Volumes |
| :--- | :---: | :---: | :---: |
| 2031 Horizon Year with <br> Posted Speed Limit <br> Reduction <br> (Derived from Region's <br> TMP Model) | 1,338 | 1,137 | 2,475 |
| 2041 Horizon Year <br> Posted Speed Limit <br> Reduction <br> (Derived from Region's <br> TMP Model) | 1,670 | 1,398 | 3,068 |
| 2031 to 2041 <br> Auto Traffic CAGR | $2 \%$ | $2 \%$ | $2 \%$ |

As a result of this analysis, a 2\% CAGR was applied to through movements to 2031 traffic volumes to forecast 2041 traffic volumes for this study. A 1\% CAGR was applied to turning movements.

A need to derive separate AM peak hour compound annual growth rates for the southbound approach at the Kennedy Road and Elgin Mills Road East intersection and southbound approach at the Warden Avenue and Elgin Mills Road East intersection was identified after review of the 2041 York Region transportation model. Due to travel

Transportation System Technical Report
June 6, 2022
patterns associated within and near the Study Areas, these approaches experience very different levels of growth. These volumes are shown in Table 40.

Table 40: 2031 and 2041 AM Modelled Traffic Volumes approaching Southbound Elgin Mills Road East at Warden Avenue and Kennedy Road

| Year | Warden Avenue: <br> Southbound <br> Approach at Elgin <br> Mills Road East | Kennedy Road: <br> Southbound <br> Approach at Elgin <br> Mills Road East |
| :--- | :---: | :---: |
| 2031 Horizon Year with 2 lanes per <br> direction north of Elgin Mills Road <br> East adjustment <br> (Derived from Region's TMP Model) | 1,244 | 1,024 |
| 2041 Horizon Year <br> (Derived from Region's TMP Model) | 1,830 | 1,147 |
| 2031 to 2041 <br> Auto Traffic CAGR | $4 \%$ | $1 \%$ |

As a result of this analysis, a 4\% CAGR was applied to the southbound through movement along Warden Avenue and a 1\% CAGR was applied to the southbound through movement along Kennedy Road in the AM peak hour.

The northbound approach at these two intersections in the PM peak hour does not need to be adjusted since the $4 \%$ screenline CAGR (see Table 39) was applied and is more conservative.

## Eastbound/Westbound Compound Annual Growth Rate Analysis

Based on the traffic growth rate analysis, the compound annual growth rates that were applied to through movements associated with mid-block volumes along Major Mackenzie Drive East and Elgin Mills Drive East between Warden Avenue and Kennedy Road was $0.5 \%$ between 2031 to 2041. A $0.25 \%$ CAGR was applied to turning movements.

### 5.4.5 2041 Traffic Volumes

The resulting 2041 traffic volumes after applying the traffic compound annual growth rates are shown in Figure 25.

Transportation System Technical Report
June 6, 2022
Figure 25: 2041 Forecasted Traffic Volumes


### 5.52041 "Do Nothing" Automobile Level of Service Evaluation

The future conditions assessment was conducted for the year 2041 using a "Do Nothing" scenario with no geometric improvements with the AM and PM peak hour demand as shown in Figure 25. The 2041 "Do Nothing" study corridors' intersection lane configuration and traffic control are shown in Figure 26.

The "Do Nothing" scenario was assessed to re-confirm the automobile needs and justification that was identified in the 2016 Transportation Master Plan. The 2016 TMP fulfills the requirements of Phases 1 and 2 of the Municipal Class Environmental Assessment process for roads. This TMP identified Kennedy Road and Warden Avenue as requiring roadway improvements which included a one to two-lane widening within the study areas. The alternative solutions and preferred solution are documented in Section 9.0 of this report.
The collector road network is not shown as future traffic operations for the Collector Roads intersecting with the study corridors were assessed in the North Markham Future Urban Area Collector Roads Network Class Environmental Assessments (see Section 2.3.4). Traffic operations assessment that includes future accesses and intersections as a result of the planned collector road system will be documented in

Transportation System Technical Report
June 6, 2022
Transportation Technical Report \#2 which assesses the traffic operations of the preferred preliminary design concept.

Although there is a planned widening on Elgin Mills Road East between Woodbine Avenue to Kennedy Road from two to four lanes between year 2027 to 2031 and a planned widening between Kennedy Road to Donald Cousens Parkway between year 2032 to 2041, this "future conditions" assessment assumed no geometric improvements at the Elgin Mills Road East intersections.

At the time of writing this report, the City of Markham was conducting a Municipal Class Environmental Assessment Study on Elgin Mills Road East between Woodbine Avenue to McCowan Road. Recommendations from this study will re-confirm the preliminary designs of the Elgin Mills Road East intersections at Kennedy Road and Warden Avenue as proposed in the City's Environmental Assessment and the needs for these York Region intersections. The compound annual traffic growth rates that were applied to traffic volumes along Elgin Mills Road East were not constraint by the existing 2-lane cross-section. This implies that traffic operations analysis incorporated the traffic demand based on a 4-lane cross-section of Elgin Mills Road East, but reports the results based on a 2-lane cross section. Unconstrained traffic growth rates are described in Section 5.4.2.

Figure 26: 2041 "Do Nothing" Lane Configuration and Traffic Control


Transportation System Technical Report
June 6, 2022

### 5.5.1 2041 Link Capacity Analysis

The $2041 \mathrm{v} / \mathrm{c}$ ratios for the study corridors were calculated for the weekday AM and PM peak hours using the 2041 forecasted traffic volumes. Congested links are assumed to have a link volume-to-capacity ratios greater than 0.90 and are highlighted in the tables below in red. Similar to the existing condition link capacity analysis, only auto volumes and auto link capacities were considered in this analysis.

The results for Warden Avenue are presented in Table 41 for the AM peak and Table 42 for the PM peak.

Table 41: Link Capacity Analysis for Warden Avenue - AM Future Conditions

| Warden | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Southbound |  |  |  |  |  |  |  |
| South of <br> Elgin Mills | 1 | 900 | 2,000 | 2.22 | 1 | 900 | 359 | 0.40 |
| Road East | 1 | 900 | 1,780 | 1.98 | 1 | 900 | 265 | 0.29 |
| North of <br> Major <br> Mackenzie <br> Drive East | 1 |  |  |  |  |  |  |  |

Table 42: Link Capacity Analysis of Warden Avenue - PM Future Conditions

| Warden <br> Avenue | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 900 | 376 | 0.42 | 1 | 900 | 1,599 | 1.78 |
| Road East |  |  |  |  |  |  |  |  |
| Sorth of <br> Major <br> Mackenzie <br> Drive East | 1 | 900 | 348 | 0.39 | 1 | 900 | 1,664 | 1.85 |

The results for Kennedy Road are presented in Table 43 for the AM peak and Table 44 for the PM peak.

Table 43: Link Capacity Analysis for Kennedy Road - AM Future Conditions

| Kennedy <br> Road | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 900 | 1,133 | 1.26 | 1 | 900 | 533 | 0.59 |
| North of <br> Major | 1 | 900 | 1,036 | 1.15 | 1 | 900 | 612 | 0.68 |

Transportation System Technical Report
June 6, 2022

| Kennedy <br> Road | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Southbound |  |  |  |  |  |  |  |  |
| Mackenzie |  |  |  |  |  |  |  |  |
| Drive East |  |  |  |  |  |  |  |  |

Table 44: Link Capacity Analysis of Kennedy Road - PM Future Conditions

| Kennedy <br> Road | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio | \# of <br> Lanes | Total <br> Capacity | Volume | v/c <br> ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Southbound |  |  |  |  |  |  |  |
| South of <br> Elgin Mills <br> Road East | 1 | 900 | 549 | 0.61 | 1 | 900 | 804 | 0.89 |
| North of <br> Major <br> Mackenzie <br> Drive East | 1 | 900 | 537 | 0.60 | 1 | 900 | 618 | 0.69 |

The link capacity analysis indicated that Warden Avenue will operate with demand above auto link capacity in the future "do nothing" conditions during the AM and PM peak hours. The maximum link volume-to-capacity ratio is 2.22 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 1.85 in the northbound direction during the PM peak hour.

The link capacity analysis indicated that Kennedy Road will operate with demand above auto link capacity in the future "do nothing" conditions during the AM peak hour. The maximum link volume-to-capacity ratio is 1.26 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 0.89 in the northbound direction during the PM peak hour.

The 2041 future year link capacity analysis showed the need for increased link capacity for Warden Avenue and Kennedy Road. Increased link capacity should be provided by increasing the number of lanes from 1 lane per direction to 2 lanes per direction as recommended by the Region's 2016 TMP.

### 5.5.2 2041 Traffic Operations - "Do Nothing" Scenario

The 2041 "Do Nothing" scenario were analyzed for the weekday AM and PM peak hours. The signalized intersection operation results for Warden Avenue and Kennedy Road are shown in Table 45. A summary of the intersection operations is shown in Figure 27. Detailed results are provided in Appendix G. Critical movements are highlighted in red.

Transportation System Technical Report
June 6, 2022
Table 45: Future Conditions Traffic Operations Analysis

| Movement | Weekday AM Peak Hour |  | Weekday PM Peak Hour |  |
| :---: | :---: | :---: | :---: | :---: |
|  | v/c | LOS | v/c | LOS |
| Warden Avenue and Elgin Mills Road East |  |  |  |  |
| Overall | 2.40 | F | 2.03 | F |
| EBL | 0.26 | B | 0.28 | B |
| EBTR | 1.25 | F | 1.99 | F |
| WBL | 1.88 | F | 0.25 | B |
| WBTR | 2.11 | F | 0.71 | B |
| NBL | 1.04 | F | 0.61 | C |
| NBT | 0.41 | B | 1.90 | F |
| NBR | 0.01 | B | 0.23 | B |
| SBL | 0.02 | B | 0.15 | B |
| SBT | 2.49 | F | 0.42 | B |
| SBR | 0.13 | B | 0.02 | B |
| Warden Avenue and Major Mackenzie Drive East |  |  |  |  |
| Overall | 1.89 | F | 1.36 | F |
| EBL | 0.02 | C | 0.16 | C |
| EBT | 0.76 | D | 1.38 | F |
| EBR | 0.10 | C | 0.11 | C |
| WBL | 0.96 | E | 0.71 | D |
| WBT | 1.23 | F | 0.42 | B |
| WBR | 0.02 | B | 0.02 | B |
| NBL | 0.50 | C | 0.78 | D |
| NBT | 0.18 | C | 1.33 | F |
| NBR | 0.05 | C | 0.59 | D |
| SBL | 0.12 | C | 0.72 | E |
| SBT | 2.77 | F | 0.61 | D |
| SBR | 0.10 | C | 0.01 | C |
| Kennedy Road and Elgin Mills Road East |  |  |  |  |
| Overall | 1.76 | F | 1.37 | F |
| EBL | 0.14 | B | 0.22 | B |
| EBT | 0.57 | B | 1.75 | F |
| EBR | 0.13 | B | 0.16 | B |
| WBL | 0.33 | B | 0.24 | B |
| WBT | 1.92 | F | 0.60 | B |
| WBR | 0.01 | B | 0.00 | B |
| NBL | 1.47 | F | 0.45 | C |
| NBT | 0.58 | B | 0.98 | D |
| NBR | 0.01 | B | 0.04 | B |
| SBL | 0.06 | B | 0.15 | B |
| SBT | 1.40 | F | 0.60 | B |
| SBR | 0.13 | B | 0.01 | B |
| Kennedy Road and Major Mackenzie Drive East |  |  |  |  |
| Overall | 1.21 | F | 1.19 | F |
| EBL | 0.28 | C | 0.17 | B |

Transportation System Technical Report
June 6, 2022

| Movement | Weekday AM Peak Hour |  | Weekday PM Peak Hour |  |
| ---: | :---: | :---: | :---: | :---: |
|  | v/c | LOS | v/c | LOS |
| EBT | 0.46 | C | 1.29 | F |
| EBR | 0.17 | C | 0.27 | B |
| WBL | 0.53 | B | 0.53 | C |
| WBT | 1.32 | F | 0.51 | C |
| WBR | 0.15 | C | 0.03 | B |
| NBL | 1.29 | F | 1.05 | F |
| NBT | 0.35 | C | 0.57 | D |
| NBR | 0.07 | C | 0.20 | C |
| SBL | 0.44 | D | 0.74 | E |
| SBT | 1.01 | F | 0.66 | D |
| SBR | 0.19 | D | 0.02 | D |

Figure 27: Summary of 2041 Future Conditions Traffic Operations Analysis


Many movements are identified as critical movements during the AM and PM peak hours under the 2041 future conditions. Similar to the existing conditions, all 4 intersections are operating well above capacity. A need for infrastructure improvements at the intersections to improve capacity and signal timing optimization is recommended to improve operations which will be addressed in the preliminary design concept.

Transportation System Technical Report
June 6, 2022

### 5.5.3 2041 Future Intersection Queues - "Do Nothing" Scenario

The 95th percentile queues for each movement at the study intersections were extracted from the Synchro 11 analysis and compared to the available storage length as shown in Table 46. Future intersection queues were highlighted in red where 95th percentile queues were higher than existing storage link distances.
Table 46: Future Conditions Queue Summary

| Movement | Existing Storage/ Link Distance (m) | Weekday AM Peak Hour | Weekday PM Peak Hour |
| :---: | :---: | :---: | :---: |
|  |  | 95th \% Queue (m) | 95th \% Queue (m) |
| Warden Avenue and Elgin Mills Road East |  |  |  |
| EBL | 70 | 8 | 13 |
| EBTR | 400+ | 198 | 350 |
| WBL | 80 | 61 | 8 |
| WBTR | 400+ | 396 | 83 |
| NBL | 125 | 42 | 43 |
| NBT | 500+ | 42 | 335 |
| NBR | 30 | 0 | 21 |
| SBL | 100 | 3 | 6 |
| SBT | 500+ | 458 | 43 |
| SBR | 30 | 13 | 1 |
| Warden Avenue and Major Mackenzie Drive East |  |  |  |
| EBL | 55 | 2 | 16 |
| EBT | 500+ | 114 | 419 |
| EBR | 85 | 16 | 17 |
| WBL | 145 | 124 | 40 |
| WBT | 450+ | 409 | 80 |
| WBR | 100 | 0 | 2 |
| NBL | 60 | 20 | 65 |
| NBT | 500+ | 31 | 340 |
| NBR | 60 | 9 | 90 |
| SBL | 140 | 17 | 30 |
| SBT | 550+ | 847 | 102 |
| SBR | 80 | 17 | 0 |
| Kennedy Road and Elgin Mills Road East |  |  |  |
| EBL | 120 | 5 | 14 |
| EBT | 350+ | 62 | 301 |
| EBR | 50 | 13 | 15 |
| WBL | 150 | 18 | 8 |
| WBT | 400+ | 331 | 65 |
| WBR | 60 | 0 | 0 |
| NBL | 125 | 49 | 26 |
| NBT | 300+ | 64 | 146 |
| NBR | 45 | 0 | 6 |
| SBL | 40 | 5 | 6 |
| SBT | 300+ | 236 | 68 |
| SBR | 35 | 13 | 0 |
| Kennedy Road and Major Mackenzie Drive East |  |  |  |
| EBL | 55 | 10 | 12 |

Transportation System Technical Report
June 6, 2022

| Movement | Existing <br> Storage/ Link <br> Distance (m) | Weekday AM Peak Hour | Weekday PM Peak Hour |
| ---: | :---: | :---: | :---: |
|  | 95th \% Queue (m) | 95th \% Queue (m) |  |
| EBT | $500+$ | 83 | 449 |
| EBR | 60 | 16 | 45 |
| WBL | 250 | 38 | 25 |
| WBT | 360 | 443 | 113 |
| WBR | 150 | 19 | 0 |
| NBL | 120 | 136 | 93 |
| NBT | $300+$ | 58 | 79 |
| NBR | 60 | 13 | 27 |
| SBL | 195 | 40 | 47 |
| SBT | $550+$ | 162 | 68 |
| SBR | 60 | 28 | 0 |

In the AM peak hour, the southbound-through forecasted queue along Warden Avenue between Major Mackenzie Drive East and Elgin Mills Road East is significantly long. The recommended improvements of widening from two to four lanes to increase the link capacity will dramatically reduce the forecasted queues.

The other traffic through movement that contained 95th percentile through movement queues long enough to impact an existing adjacent signalized intersection was westbound at Kennedy Road and Major Mackenzie Drive East.

Major Mackenzie Drive East is planned for a widening from 4 lanes to 6 lanes between Leslie Street to Kennedy Road to accommodate transit and high-occupancy-vehicle lanes. Note that this widening has been identified in the Region's long-term plan through the 2016 TMP and has not been included in the 2022 10-Year Roads and Transit Capital Construction Program. Signal timing optimization and coordination is recommended to mitigate spillback at the adjacent intersections.

The forecasted 95th percentile northbound left queue is greater than the existing storage distance at Kennedy Road and Major Mackenzie Drive East. Infrastructure improvements (ex. Dual left turns), increasing left turn storage distances, signal timing optimization, or lane configuration alterations will be reviewed to mitigate the impacts. This will be addressed in the preferred preliminary design concept.

Transportation System Technical Report
June 6, 2022

### 6.0 Signal Warrant Analysis

All 4 major intersections within the study corridors are already signalized.
The City of Markham's Future Urban Area Conceptual Master Plan recommended a comprehensive collector road network consisting of roads and active transportation infrastructure. This network is shown in Figure 28. The signal warrant analysis undertaken in WSP's Robinson Glen Traffic Impact Study Update (see Section 3.2) concluded the following:

- Kennedy Road at EW-1 and EW-3 are expected to approach being warranted by year 2031.
- Kennedy Road at L-3 was assumed unsignalized due to its short segment and less direct connectivity.

Figure 28: FUA Collector Road Network

Adapted from: Future Urban Area Conceptual Master Plan Volume 2 (October 2018)


The Angus Glen Block Collector Road Class Environmental Assessment is expected to confirm signalization of the FUA collector road network along Warden Avenue. This EA is currently on hold due to Ontario Land Tribunal hearings. The FUA Conceptual Master Plan Volume 2 proposed signals along Warden Avenue at EW-3, EW-2, and EW-1. The traffic analysis that will be undertaken for the preliminary design concept will confirm the

Transportation System Technical Report June 6, 2022
signalization of intersections of the Collector Roads and will be documented in Transportation Technical Report \#2.

Transportation System Technical Report
June 6, 2022

### 7.0 Roundabout Screening

There are a few general criteria that may merit consideration of roundabout implementation. These criteria include:

- Locations where there are capacity or safety problems at an existing intersection. Roundabouts may provide greater capacity than traffic signals or stop controlled intersections. Overall safety is also improved by slower intersection speeds and by reducing the number of conflict points.
- Where traffic signals or all-way stops are warranted or expected to be warranted in the near future at existing or proposed intersections.
- Where, as part of a larger capital project, suitable intersections are identified as potential sites. There is a potential significant cost savings associated with roundabouts as planned roadway re-construction usually requires removal of existing asphalt, granular material, curbing, sidewalks, etc.
- Where, through the development process, new intersections are introduced.

Although an alternative to signalization, roundabouts have an upper limit on the volume of traffic that can be accommodated through its intersection. Based on the U.S Federal Highway Administration's "Roundabout: An Informational Guide" (2000), single-lane roundabouts can generally be considered when circulating flow is less than 1,800 vehicles per hour or AADT flow less than 25,000 vehicles. A double-lane roundabout can generally be considered when circulating flow is less than 3,000 vehicles/hour pr AADT flow less than 34,000 vehicles. Multi-lane roundabouts with more than two lanes are not recommended in the context of York Region within the 2041 planning horizon. Given that they are more complicated for drivers, multi-lane roundabouts are rare in York Region and surrounding areas. There are no roundabouts with three or more lanes yet in Ontario.

Careful consideration should be made for vulnerable road users such as cyclists and pedestrians. Roundabouts are more difficult for cyclists and pedestrians to cross and may require pedestrian crossings. Careful consideration should also be made by those who are mobility challenged and require accessibility considerations. When the roundabout becomes busy, the approach entrances and exit lanes could become intimidating for this population, which typically require longer gaps in the traffic stream to safely cross.

Given these general considerations, this study does not recommend including the implementation of roundabouts as an alternative to be considered for the Major Mackenzie Drive East and Elgin Mills Road East intersections for the following reasons:

- Given the forecasted 2041 traffic volumes as shown in Figure 25 (see Section 5.4.5), a 3-lane roundabout would have to be considered. A 3-lane roundabout is not an appropriate solution in the context of York Region, and specifically in the Future Urban Area where in an urbanized environment, comfort for vulnerable road users and those that have mobility challenges should be prioritized.

Transportation System Technical Report June 6, 2022

- These intersections are not shown to have safety problems as shown in the traffic safety assessment (see Section 4.7). Although single-lane roundabouts can improve safety, a multi-lane roundabout may not have the same level of safety benefits.
- The four intersections within the study corridors are already signalized and generally roundabout implementation should be considered for new or unsignalized intersections.

Transportation System Technical Report
June 6, 2022

### 8.0 Summary of Needs and Justifications

An existing and future transportation assessment and safety analysis was undertaken for the Warden Avenue and Kennedy Road corridors between Major Mackenzie Drive East and Elgin Mills Road East. Based on the analysis, a summary of the needs and justification are provided below:

## Automobile Needs and Justification:

In the 2021 existing conditions year:

- During the AM peak hour, the traffic operations analysis indicated three intersections operating above capacity. Warden Avenue and Elgin Mills Road East operate at LOS E. Warden Avenue and Major Mackenzie Drive East operate at LOS F. Kennedy Road and Major Mackenzie Drive East operate at LOS E.
- During the PM peak hour, the traffic operations indicated one intersection operating above capacity. Warden Avenue and Major Mackenzie Drive East operate at LOS E.
- The link capacity analysis indicated that Warden Avenue is operating above auto link capacity during both AM and PM peak hours. The maximum auto link volume-to-capacity ratio was 1.24 in the southbound direction during the AM peak hour. The maximum auto link volume-to-capacity ratio was 1.15 in the northbound direction during the PM peak hour.
- This link capacity analysis indicated that Kennedy Road is operating below total link capacity during both AM and PM peak hours. The maximum link volume-to-capacity ratio is 0.78 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 0.58 in the northbound direction during the PM peak hour.

In the 2041 future conditions year for the "Do Nothing" assessment:

- During the AM and PM peak hours, the traffic operations analysis indicated that all 4 intersections was forecasted to operate above capacity at LOS F indicating a need for intersection improvements.
- In the AM peak hour, the southbound-through forecasted queue along Warden Avenue between Major Mackenzie Drive East and Elgin Mills Road East is significantly long. The recommended improvements of widening from two to four lanes to increase the link capacity will dramatically reduce the forecasted queues.
- The other movement that was forecasted to have 95th percentile through movement queues that was long enough to impact an existing adjacent signalized intersection was westbound at Kennedy Road and Major Mackenzie Drive East. Signal timing optimization and coordination is recommended to mitigate spillback at the adjacent intersections.
- The 95th percentile queue is forecasted to be greater than the existing northbound left storage distance at Kennedy Road and Major Mackenzie Drive East. Infrastructure improvements (e.g., Dual left turns), increasing left turn storage

Transportation System Technical Report
June 6, 2022
distances, signal timing optimization, or lane configuration alterations are recommended to be explored to mitigate the impacts.

- The link capacity analysis indicated that Warden Avenue is forecasted to operate above auto link capacity in the future "do nothing" scenario during the AM and PM peak hours. The maximum link volume-to-capacity ratio is 2.22 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 1.85 in the northbound direction during the PM peak hour.
- The link capacity analysis indicated that Kennedy Road is forecasted to operate above auto link capacity in the future "do nothing" scenario during the AM peak hour. The maximum link volume-to-capacity ratio is 1.26 in the southbound direction during the AM peak hour. The maximum link volume-to-capacity ratio is 0.89 in the northbound direction during the PM peak hour.
- The 2041 future year link capacity analysis indicated the need for increased link capacity for Warden Avenue and Kennedy Road. Increased link capacity should be provided by increasing the number of lanes from one lane per direction to two lanes per direction as recommended by the Region's 2016 TMP.


## Pedestrian and Cycling Needs and Justification

- There are no existing dedicated cycling facilities along Warden Avenue or Kennedy Road within the Study Areas.
- Pedestrian Levels-of-Service did not meet the target of LOS C for many legs of intersections and roadway segments within the study corridors under the existing conditions.
- Improvements to the pedestrian and cycling environment should be implemented at the intersections of Elgin Mills Road East at Warden Avenue and Kennedy Road. Improvements to the pedestrian environment should be implemented on Warden Avenue and Kennedy Road between Elgin Mills Road East and Major Mackenzie Drive East.
- Because the area is being redeveloped and reconstructed, opportunities to improve the pedestrian and cycling environment improvement should be explored at the Major Mackenzie Drive East intersections at Warden Avenue and Kennedy Road concurrently with the improvements to the rest of the study corridors to ensure consistency and connectivity.
- There is an existing multi-use path along Warden Avenue between 16 th Avenue and Major Mackenzie Drive East on the east side. Given that the area is planned for redevelopment with an urban cross section for Warden Avenue, it would be appropriate to continue the multi-use path north of Major Mackenzie Drive East on the east side for connectivity of the active transportation network.


## Transit Needs and Justification

- There is no transit service currently provided within the Study Areas. The Region supports transit as a robust transit network helps support growth to key centres and corridors and minimizes the need for travel and reduces dependence on single

Transportation System Technical Report
June 6, 2022
occupant vehicles. This sustainable mode of transportation also helps mitigate climate change, which is another important objective for York Region.

- Warden Avenue and Kennedy Road between Elgin Mills Road East and Major Mackenzie Drive East are planned to be frequent transit network routes.


## Traffic Safety Needs and Justification

- A safety assessment was undertaken to identify and mitigate potential safety related concerns using historical collision records within the Study Areas.
- Warden Avenue between Major Mackenzie Drive East and Heritage Hill Drive has collision rate of 1.80 along this segment indicating a need for safety improvements.
- At the Kennedy Road and Elgin Mills Road East intersection, 47\% of the collisions were categorized as non-fatal and $53 \%$ was categorized as property damage only. The severity of the collisions may represent an opportunity for improvement, such as speed management.
- There were 7 single motor vehicle (SMV) collisions out of 10 along Kennedy Road between Elgin Mills Road East and Major Mackenzie Drive East accounting for $70 \%$ of the collisions along that segment between year 2015 to 2019. Four of the seven were SMV collisions involved an animal collision
- The alternative design concepts should consider the need to protect for vulnerable road users since active transportation activity is expected to increase along this corridor.


## Roundabout Needs and Justification

- This study does not recommend including the implementation of roundabouts as an alternative to be considered for both study corridors at the Major Mackenzie Drive East and Elgin Mills Road East intersections based on the following reasons:
- Given the forecasted 2041 traffic volumes, a 3-lane roundabout would have to be considered. A 3-lane roundabout is not recommended in York Region currently, and specifically in the Future Urban Area where in an urbanized environment, comfort for vulnerable road users and those that have mobility challenges should be prioritized.
- Roundabouts should be considered if there are safety concerns at the intersections. However, these intersections are not shown to have safety problems as shown in the traffic safety assessment. Although single-lane roundabouts can improve safety, a multi-lane roundabout may not have the same level of safety benefits.
- The 4 intersections within the study corridors are already signalized and generally roundabout implementation should be considered for new or unsignalized intersections.

Transportation System Technical Report
June 6, 2022

### 9.0 Alternative Solutions

### 9.1 Overview

York Region's Transportation Master Plan (TMP) is the Region's long-term vision for York Region's transportation network, encompassing strategy, policy, and infrastructure. The plan looks ahead 30 years and considers the Region's transportation infrastructure needs to support growth and the changing needs of travellers. The plan supports healthy communities and economic growth by planning for safe, reliable travel, and efficient movement of goods.

The TMP was last conducted in 2016 and satisfies Phases 1 and 2 of the MCEA process. The TMP identified Warden Avenue and Kennedy Road as requiring roadway improvements. These recommendations were based on assessments of alternative scenarios. A preferred scenario was selected based on its alignment with the following TMP objectives:

- Support Transit
- Support Road Network.
- Support Active Transportation.
- Support Goods Movement.
- Support Last Mile.

An overview of the problem and opportunity, alternative scenarios, and preferred scenario for each study corridor is outlined in the below sections.

### 9.2 Warden Avenue between Major Mackenzie Drive and Donald Cousens Parkway (Project ID 2073)

The TMP identified Warden Avenue between Major Mackenzie Drive to Donald Cousens Parkway (Project ID 2073) as requiring roadway improvement. The forecasted 2041 Do Nothing average modelled traffic volume was 2,000 vehicles per hour in the peak direction. This modelled traffic volume results in an average modelled volume-tocapacity ( $\mathrm{v} / \mathrm{c}$ ) ratio of 1.81 . This $\mathrm{v} / \mathrm{c}$ ratio suggests a road widening is required to mitigate congestion.

The 2016 TMP proposed a 4-lane road throughout this section. This proposed lane configuration along Warden Avenue resulted in an average modelled traffic volume of 2,610 vehicles per hour in 2041. This modelled traffic volume resulted in an average v/c ratio of 1.36 .

The problem and opportunity statement included the following:

- Transportation network improvements are needed to accommodate expansion of the Designated Urban Area.
- Capacity improvements needed to accommodate future travel demands.

Transportation System Technical Report
June 6, 2022

- Corridor improvements needed to support walking and cycling.
- Corridor improvements need to support transit.

Alternatives that were considered and the results are shown in Table 47.

## Table 47: Alternative Scenario Analysis for Warden Avenue (2016 York Region TMP)

| Scenario \# | Description | Result |
| :---: | :--- | :--- |
| 1 | Do Nothing | Did not address the problem or opportunity <br> statement. |
| 2 | Optimize existing facility <br> with intersection <br> improvements only. | Provided minor improvements to traffic flow. <br> Did not address overall traffic congestion. |
| 3 | Urbanize corridor but <br> maintain 2-lane <br> cross-section. | Did not address traffic congestion. Addressed <br> opportunity to improve walking and cycling <br> facilities. |
| 4 | Widen corridor to 4 <br> lanes and maintain rural <br> cross-section. | Addressed traffic capacity. Did not address. <br> opportunity to improve walking and cycling <br> facilities. |
| 5 | Widen corridor to 4 <br> lanes and construct to <br> urban cross-section. | Addressed traffic capacity. Addressed <br> opportunity to improve walking and cycling <br> facilities. |
| 6 | Widen parallel/adjacent <br> corridor. | Potential to divert some traffic to other <br> corridors. Did not address corridor <br> congestion and provided no improvements to <br> walking and cycling facilities. |

The 2016 TMP recommended widening the corridor to 4 lanes and constructing to an urban arterial standard. The justification provided was that the traffic forecast met threshold for a 4-lane widening. This recommendation provided an opportunity to improve walking and cycling facilities. The TMP recommended the widening from Major Mackenzie Drive East to Elgin Mills Road East by 2031.

### 9.3 Kennedy Road between Major Mackenzie Drive and Donald Cousens Parkway (Project ID 2003)

The TMP identified Kennedy Road between Major Mackenzie Drive to Donald Cousens Parkway (Project ID 2003) as requiring roadway improvement. The forecasted 2041 Do Nothing average modelled traffic volume was 1,430 vehicles per hour in the peak direction. This modelled traffic volume results in an average modelled volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio of 1.42 . This $\mathrm{v} / \mathrm{c}$ ratio suggests a road widening is required to mitigate congestion.

The 2016 TMP proposed a 4-lane road throughout this section. This proposed lane configuration along Kennedy Road resulted in an average modelled traffic volume of 1,760 vehicles per hour in 2041. This modelled traffic volume resulted in an average v/c ratio of 0.88 .

Transportation System Technical Report
June 6, 2022
The problem and opportunity statement included the following:

- Transportation network improvements are needed to accommodate expansion of the Designated Urban Area.
- Capacity improvements needed to accommodate future travel demands.
- Corridor improvements needed to support walking and cycling.

Alternatives that were considered and the results are shown in Table 48.
Table 48: Alternative Scenario Analysis for Kennedy Road (2016 York Region TMP)

| Scenario \# | Description | Result |
| :---: | :--- | :--- |
| 1 | Do Nothing | Did not address the problem or opportunity <br> statement. |
| 2 | Optimize existing facility <br> with intersection <br> improvements only. | Provided minor improvements to traffic flow. <br> Did not address overall traffic congestion. |
| 3 | Urbanize corridor but <br> maintain 2-lane cross- <br> section. | Did not address traffic congestion. <br> Addressed opportunity to improve walking <br> and cycling facilities. |
| 4 | Widen corridor to 4 lanes <br> and construct to urban <br> arterial standard. | Addressed traffic capacity. Addressed <br> opportunity to improve walking and cycling <br> facilities. |
| 5 | Widen parallel/adjacent <br> corridor.Potential to divert some traffic to other <br> corridors. Did not address corridor <br> congestion and provided no improvements <br> to walking and cycling facilities. |  |

The 2016 TMP recommended widening of the corridor from two lanes to four lanes and constructing to an urban arterial standard. The justification provided was that the traffic forecast met threshold for a 4-lane widening. This recommendation provided an opportunity to improve walking and cycling facilities. The TMP recommended the widening from Major Mackenzie Drive East to Elgin Mills Road East by 2031.

# Burnside 

[The Difference is our People]

# Appendix A 

Turning Movement Counts and Supplemental Traffic Data

## AADT Midblocks Report

| Description | 2010 | 2012 | 2014 | 2015 | 2017 | 2019 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Kennedy Road btwn Major Mackenzie Drive East \& Elgin Mills Road East | 6913 | 6907 | 7318 | 7283 | 7910 | 6972 |

## AADT Midblocks Report

| Description | 2010 | 2012 | 2014 | 2016 | 2018 |
| :---: | :---: | :---: | :---: | :--- | :--- |
| Warden Avenue btwn Major Mackenzie Drive East \& Heritage Hill Drive | 6869 | 6928 | 7339 | 11811 | 11481 |

## AADT Midblocks Report

| Description | 2010 | 2012 | 2014 | 2015 | 2017 | 2019 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Warden Avenue btwn Heritage Hill Drive \& Elgin Mills Road East | 5257 | 5347 | 5656 | 5754 | 6234 | 6014 |

## Turning Movements Diagram <br> Peak Hour Report: AM Period



Notes:

## Turning Movements Diagram <br> Peak Hour Report: MD Period



Notes:

## Turning Movements Diagram <br> Peak Hour Report: PM Period



Notes:

# Turning Movements Diagram <br> Peak Hour Report: AM Period 

Location....... Kennedy Road \& Major Mackenzie Drive East
GeoID....... DEA43F1B
Municipality. Markham
Count Date. Wednesday, 22 March, 2017
Traffic Cont. Traffic signal
Count Period. 07:00 AM - 09:00 AM
Major Dir..... None
Peak Hour.... 07:45 AM - 08:45 AM


Notes:

## Turning Movements Diagram <br> Peak Hour Report: MD Period



Notes:

## Turning Movements Diagram <br> Peak Hour Report: PM Period

Location....... Kennedy Road \& Major Mackenzie Drive East

Municipality. Markham
Traffic Cont. Traffic signal
Major Dir..... None

GeoID.......
DEA43F1B


Notes:

## Turning Movements Diagram <br> Peak Hour Report: AM Period

| Location....... Warden Avenue \& Elgin Mills Road East | GeoID....... | 4747A62F |
| :--- | :--- | :--- |
| Municipality. Markham | Count Date. | Wednesday, 11 November, <br> 2009 |
| Traffic Cont. Traffic signal | Count Period. 07:00 AM - 09:00 AM |  |
| Major Dir..... None | Peak Hour.... 07:30 AM - 08:30 AM |  |



Notes:

## Turning Movements Diagram <br> Peak Hour Report: MD Period



Notes:

## Turning Movements Diagram <br> Peak Hour Report: PM Period



Notes:

## Turning Movements Diagram <br> Peak Hour Report: AM Period



Notes:

## Turning Movements Diagram <br> Peak Hour Report: MD Period



Notes:

## Turning Movements Diagram <br> Peak Hour Report: PM Period



Notes:

## (1) Burnside

North Markham Future Urban Area Conceptual Master Plan - Transportation Study Traffic Model Calibration and Validation, Parsons (December 10, 2014)

Existing Turning Movement Counts (Balanced), Page 9:


## (1) Burnside

North Markham Future Urban Area Conceptual Master Plan - Transportation Study Existing Conditions Analysis, Parsons (March 13, 2015)

2031 AM Peak Hour Traffic Volumes within the FUA, Page 18


Figure 16-2031 AM Peak Hour Traffic Volumes within the FUA Burnside
[The Difference is our People]

## Appendix B

Signal Timing Plans



 Burnside
[The Difference is our People]

## Appendix C

## Speed Studies

Date Start: 12-Jun-19
Date End: 12-Jun-19


| $\begin{array}{r} \mathrm{Gr} \\ \text { Total } \end{array}$ | 2 | 2 | 7 | 12 | 22 | 38 | 42 | 53 | 66 | 113 | 242 | 666 | 911 | 1166 | 969 | 473 | 225 | 73 | 29 | 8 | 5 | 6 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 5131 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Stats |  |  |  |  | 15th P | entile : |  |  | 67 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 50th P | entile : |  |  | 77 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 85th P | entile : |  |  | 86 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 95th P | entile : |  |  | 92 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | Mean | Speed( | rage) : |  |  | 76 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Numb | of Ve | icles > | KPH : |  |  | 4774 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Perce | of Ve | icles > | KPH : |  |  | 93.0\% |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Date Start: 12-Jun-19
Date End: 12-Jun-19


| $\begin{array}{r} \mathrm{Gr} \\ \text { Total } \end{array}$ | 0 | 0 | 2 | 8 | 16 | 34 | 51 | 36 | 18 | 40 | 72 | 210 | 466 | 897 | 1208 | 1131 | 673 | 359 | 183 | 50 | 18 | 5 | 7 | 3 | 1 | 1 | 0 | 1 | 0 | 0 | 5490 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Stats |  |  |  |  | 15th P | entile : |  |  | 74 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 50th P | entile : |  |  | 84 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 85th P | entile : |  |  | 94 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 95th P | ntile : |  |  | 100 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | Mean | Speed( | rage) : |  |  | 83 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Num | of V | hicles > | KPH : |  |  | 5285 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Perc | of Ve | hicles > | KPH : |  |  | 96.3\% |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Date Start: 12-Jun-19
Date End: 12-Jun-19

| NB, SB |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | St | 12- | 19 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start | 1 | 16 | 21 | 26 | 31 | 36 | 41 | 46 | 51 | 56 | 61 | 66 | 71 | 76 | 81 | 86 | 91 | 96 | 101 | 106 | 111 | 116 | 121 | 126 | 131 | 136 | 141 | 146 | 151 | 156 |  |
| Time | 15 | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | 65 | 70 | 75 | 80 | 85 | 90 | 95 | 100 | 105 | 110 | 115 | 120 | 125 | 130 | 135 | 140 | 145 | 150 | 155 | 9999 |  |
| 06/12 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 119 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 4 | 2 | 5 | 9 | 7 | 8 | 4 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 42 |
| 01:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 3 | 3 | 0 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 |
| 02:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 |
| 03:00 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 |
| 04:00 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 3 | 2 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 |
| 05:00 | 0 | 0 | 0 | 1 | 3 | 8 | 9 | 8 | 0 | 2 | 3 | 6 | 7 | 15 | 10 | 12 | 15 | 11 | 5 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 119 |
| 06:00 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 1 | 3 | 6 | 6 | 26 | 46 | 56 | 73 | 57 | 34 | 5 | 6 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 326 |
| 07:00 | 0 | 0 | 0 | 0 | 1 | 0 | 5 | 2 | 5 | 12 | 15 | 47 | 97 | 188 | 256 | 202 | 120 | 56 | 28 | 6 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1041 |
| 08:00 | 0 | 1 | 0 | 2 | 1 | 4 | 13 | 17 | 16 | 20 | 25 | 115 | 117 | 196 | 189 | 146 | 81 | 47 | 22 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1014 |
| 09:00 | 0 | 0 | 1 | 2 | 5 | 9 | 2 | 4 | 2 | 5 | 18 | 39 | 78 | 125 | 149 | 120 | 65 | 28 | 14 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 668 |
| 10:00 | 0 | 0 | 1 | 1 | 2 | 5 | 1 | 5 | 7 | 6 | 17 | 30 | 69 | 101 | 121 | 89 | 49 | 22 | 13 | 2 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 542 |
| 11:00 | 0 | 1 | 1 | 1 | 6 | 8 | 7 | 14 | 5 | 7 | 25 | 53 | 73 | 97 | 101 | 78 | 39 | 16 | 4 | 4 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 543 |
| 12 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| PM | 0 | 0 | 0 | 1 | 0 | 6 | 3 | 3 | 4 | 2 | 7 | 47 | 76 | 111 | 99 | 88 | 21 | 13 | 9 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 491 |
| 13:00 | 0 | 0 | 0 | 4 | 5 | 4 | 10 | 8 | 8 | 11 | 13 | 46 | 67 | 115 | 83 | 76 | 33 | 9 | 3 | 4 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 501 |
| 14:00 | 0 | 0 | 0 | 0 | 2 | 2 | 9 | 1 | 8 | 11 | 12 | 39 | 105 | 147 | 93 | 69 | 38 | 18 | 5 | 4 | 2 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 567 |
| 15:00 | 0 | 0 | 0 | 2 | 2 | 3 | 5 | 5 | 16 | 21 | 48 | 95 | 119 | 150 | 145 | 95 | 51 | 22 | 8 | 4 | 0 | 3 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 796 |
| 16:00 | 0 | 0 | 1 | 2 | 1 | 8 | 3 | 4 | 2 | 23 | 48 | 106 | 128 | 196 | 202 | 111 | 46 | 18 | 7 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 910 |
| 17:00 | 1 | 0 | 4 | 1 | 5 | 5 | 8 | 9 | 4 | 21 | 35 | 64 | 110 | 164 | 185 | 131 | 63 | 23 | 14 | 5 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 854 |
| 18:00 | 0 | 0 | 0 | 2 | 1 | 3 | 7 | 2 | 3 | 0 | 15 | 66 | 85 | 141 | 147 | 72 | 62 | 26 | 11 | 2 | 1 | 1 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 650 |
| 19:00 | 0 | 0 | 0 | 0 | 1 | 1 | 3 | 1 | 1 | 1 | 8 | 27 | 83 | 90 | 114 | 82 | 50 | 21 | 8 | 3 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 497 |
| 20:00 | 0 | 0 | 0 | 1 | 1 | 3 | 2 | 3 | 0 | 6 | 8 | 26 | 71 | 86 | 121 | 75 | 37 | 13 | 8 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 464 |
| 21:00 | 0 | 0 | 0 | 0 | 1 | 1 | 4 | 2 | 1 | 0 | 6 | 44 | 48 | 63 | 50 | 54 | 28 | 11 | 6 | 1 | 2 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 324 |
| 22:00 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 3 | 5 | 11 | 23 | 32 | 32 | 21 | 9 | 8 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 149 |
| 23:00 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 5 | 9 | 11 | 16 | 15 | 7 | 8 | 5 | 2 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 84 |
| Total | 2 | 2 | 9 | 20 | 38 | 72 | 93 | 89 | 84 | 153 | 314 | 876 | 1377 | 2063 | 2177 | 1604 | 898 | 432 | 212 | 58 | 23 | 11 | 7 | 4 | 1 | 1 | 0 | 1 | 0 | 0 | $\begin{array}{r} 1062 \\ 1 \end{array}$ |
| $\begin{gathered} \text { Perc } \\ \text { ent } \\ \hline \end{gathered}$ | 0.0\% | 0.0\% | 0.1\% | 0.2\% | 0.4\% | 0.7\% | 0.9\% | 0.8\% | 0.8\% | 1.4\% | 3.0\% | 8.2\% | $\begin{array}{r} 13.0 \\ \% \\ \hline \end{array}$ | $\begin{array}{r} 19.4 \\ \% \end{array}$ | $\begin{array}{r} 20.5 \\ \% \\ \hline \end{array}$ | $\begin{array}{r} 15.1 \\ \% \end{array}$ | 8.5\% | 4.1\% | 2.0\% | 0.5\% | 0.2\% | 0.1\% | 0.1\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% |  |



| $\begin{array}{r} \mathrm{Gr} \\ \text { Total } \end{array}$ | 2 | 2 | 9 | 20 | 38 | 72 | 93 | 89 | 84 | 153 | 314 | 876 | 1377 | 2063 | 2177 | 1604 | 898 | 432 | 212 | 58 | 23 | 11 | 7 | 4 | 1 | 1 | 0 | 1 | 0 | 0 | 1062 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Stats |  |  |  |  | 15th P | entile : |  |  | 70 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 50th P | entile : |  |  | 81 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 85th P | entile : |  |  | 91 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  | 95th P | entile : |  |  | 98 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | Mean | Speed( | rage) : |  |  | 80 KPH |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Numb | of Ve | hicles > | KPH: |  |  | 10059 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | Perce | of Ve | hicles > | KPH : |  |  | 94.7\% |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Location: Warden Avenue btwn Major Mackenzie Drive East \& Heritage Hill Drive
Municipality: Markham
Direction: Southbound
Date: 19-Jun-18
Speed Limit: 50 Compliance: 1\%

|  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Hour | 0-15 | 16-20 | 21-25 | 26-30 | 31-35 | 36-40 | 41-45 | 46-50 | 51-55 | 56-60 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| 6 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 6 | 4 |
| 7 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 5 | 9 | 24 |
| 8 | 0 | 0 | 0 | 0 | 1 | 0 | 2 | 11 | 20 | 32 |
| 9 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 3 | 23 | 27 |
| 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 7 | 13 |
| 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 6 |
| 12 | 0 | 0 | 0 | 0 | 2 | 0 | 1 | 0 | 2 | 11 |
| 13 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 6 |
| 14 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 5 | 5 |
| 15 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 2 | 6 | 13 |
| 16 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 9 |
| 17 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 3 | 10 |
| 18 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 4 | 5 |
| 19 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 5 | 4 |
| 20 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 2 |
| 21 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 5 |
| 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 23 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grand Total | 0 | 0 | 0 | 0 | 4 | 4 | 11 | 33 | 101 | 178 |


|  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Hour | $0-15$ | $16-20$ | $21-25$ | $26-30$ | $31-35$ | $36-40$ | $41-45$ | $46-50$ | $51-55$ | $56-60$ |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |


| 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 3 |
| 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 6 |
| 7 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 14 |
| 8 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 5 | 75 |
| 9 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 3 | 59 |
| 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 31 |
| 11 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 25 |
| 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 26 |
| 13 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 17 |
| 14 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 14 |
| 15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 29 |
| 16 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 53 |
| 17 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 63 |
| 18 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 39 |
| 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 24 |
| 20 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 |
| 21 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 17 |
| 22 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 6 |
| 23 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 13 |
| Grand Total | 3 | 0 | 0 | 0 | 0 | 0 | 3 | 5 | 53 | 543 |

Monday, January 20, 2020

## Range

| Average: | 74.08 | $\mathrm{~km} / \mathrm{h}$ |
| :--- | :--- | :--- |
| Median: | 74.55 | $\mathrm{~km} / \mathrm{h}$ |
| 85th\% | 81.98 | $\mathrm{~km} / \mathrm{h}$ |
| 95th\% | 86.41 | $\mathrm{~km} / \mathrm{h}$ |


| 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 61-65 | 66-70 | 71-75 | 76-80 | 81-85 | 86-90 | 91-95 | 96-100 | 101-105 | 106-110 | 111-115 | 116-120 |
| 0 | 0 | 8 | 8 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0 | 2 | 9 | 7 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0 | 2 | 4 | 5 | 2 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0 | 0 | 9 | 4 | 5 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0 | 1 | 2 | 10 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0 | 5 | 25 | 17 | 9 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 12 | 44 | 71 | 63 | 31 | 14 | 1 | 0 | 0 | 0 | 0 | 0 |
| 67 | 134 | 284 | 240 | 128 | 55 | 4 | 0 | 0 | 0 | 0 | 0 |
| 104 | 179 | 335 | 309 | 110 | 42 | 2 | 0 | 0 | 0 | 0 | 0 |
| 61 | 161 | 216 | 184 | 94 | 30 | 3 | 2 | 0 | 0 | 0 | 0 |
| 27 | 45 | 74 | 58 | 32 | 9 | 1 | 0 | 0 | 0 | 0 | 0 |
| 12 | 27 | 45 | 45 | 21 | 11 | 0 | 0 | 0 | 0 | 0 | 0 |
| 14 | 25 | 48 | 42 | 19 | 4 | 1 | 0 | 0 | 0 | 0 | 0 |
| 16 | 29 | 51 | 51 | 17 | 10 | 2 | 1 | 0 | 0 | 0 | 0 |
| 28 | 29 | 47 | 46 | 17 | 12 | 0 | 0 | 0 | 0 | 0 | 0 |
| 66 | 45 | 123 | 109 | 55 | 29 | 3 | 1 | 0 | 0 | 0 | 0 |
| 34 | 51 | 143 | 116 | 62 | 34 | 6 | 3 | 0 | 0 | 0 | 0 |
| 45 | 64 | 125 | 155 | 91 | 40 | 2 | 2 | 0 | 0 | 0 | 0 |
| 32 | 52 | 116 | 131 | 64 | 28 | 1 | 0 | 0 | 0 | 0 | 0 |
| 16 | 20 | 72 | 57 | 54 | 20 | 1 | 0 | 0 | 0 | 0 | 0 |
| 10 | 17 | 36 | 28 | 19 | 5 | 1 | 1 | 0 | 0 | 0 | 0 |
| 8 | 12 | 24 | 31 | 12 | 7 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4 | 9 | 27 | 26 | 6 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 3 | 8 | 19 | 16 | 7 | 6 | 0 | 1 | 0 | 0 | 0 | 0 |
| 559 | 961 | 1913 | 1758 | 872 | 368 | 28 | 11 | 0 | 0 | 0 | 0 |


| 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $61-65$ | $66-70$ | $71-75$ | $76-80$ | $81-85$ | $86-90$ | $91-95$ | $96-100$ | $101-105$ | $106-110$ | $111-115$ | $116-120$ |
| 4 | 5 | 2 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |


| 10 | 6 | 3 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5 | 2 | 7 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 10 | 8 | 0 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 3 | 1 | 16 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 11 | 11 | 3 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 11 | 18 | 15 | 7 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 |
| 61 | 86 | 64 | 17 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 167 | 128 | 63 | 17 | 4 | 1 | 2 | 0 | 0 | 0 | 0 | 0 |
| 172 | 108 | 21 | 9 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 73 | 59 | 34 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 88 | 50 | 29 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 103 | 55 | 15 | 5 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 69 | 87 | 34 | 10 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 95 | 94 | 55 | 15 | 4 | 2 | 1 | 0 | 0 | 0 | 0 | 0 |
| 183 | 212 | 161 | 40 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| 340 | 479 | 217 | 77 | 13 | 5 | 1 | 0 | 0 | 0 | 0 | 0 |
| 293 | 427 | 226 | 78 | 11 | 3 | 1 | 0 | 0 | 0 | 0 | 0 |
| 250 | 261 | 160 | 36 | 8 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 79 | 92 | 51 | 14 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 59 | 54 | 24 | 5 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |
| 49 | 41 | 18 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 43 | 32 | 8 | 3 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 22 | 28 | 5 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2200 | 2344 | 1231 | 351 | 67 | 21 | 6 | 0 | 0 | 0 | 0 | 0 |

Page 1 of 1

| 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | Grand Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 121-125 | 126-130 | 131-135 | 136-140 | 141-145 | 145-150 | 151-155 | 156-160 |  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 24 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 16 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 19 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 19 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 61 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 248 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 953 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1147 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 807 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 269 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 171 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 169 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 185 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 191 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 455 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 462 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 539 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 434 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 250 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 122 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 101 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 76 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 60 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6801 |


| 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | Grand Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $121-125$ | $126-130$ | $131-135$ | $136-140$ | $141-145$ | $145-150$ | $151-155$ | $156-160$ |  |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 |


| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 24 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 21 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 30 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 62 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 251 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 465 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 377 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 204 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 201 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 208 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 222 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 281 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 635 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1189 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1106 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 764 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 266 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 167 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 134 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 95 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 71 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6827 | Burnside

[The Difference is our People]

## Appendix D

Collision Data

Collision Details Report
From: January 1, 2010 To: April 30, 2021

| Location $\qquad$ Kennedy Road \& Elgin Mills Road East Traffic Control.... Traffic signal |  |  |  |  |  |  | Municipality........ Markham <br> Total Collisions.... 34  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Collision ID | Date/Day/Time | Environment | Impact Type | Classification | Direction | Surface Cond'n | Vehicle Manoeuver | Vehicle type | First Event | Driver Action | No. Ped |
| 20-69060 | 2020-Feb-24, Mon,16:15 | Clear | Rear end | P.D. only | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Other |  |
| Comments: |  |  |  |  | South | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 20-52964s | 2020-Feb-12, Wed, 10:00 | Clear | Sideswipe | P.D. only | East | Dry | Changing lanes | Truck - tractor | Other motor vehicle | Improper lane change |  |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 20-400925 | 2020-Dec-12, Sat, 14:04 | Rain | Angle | Non-fatal injury | West | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |  |
| Comments: |  |  |  |  | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 20-342048 | 2020-Oct-23, Fri, 18:49 | Rain | Rear end | P.D. only | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |  |
| Comments: |  |  |  |  | South | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 19-48923s | 2019-Feb-12, Tue, 10:15 | Snow | Angle | P.D. only | West | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Speed too fast for condition |  |
| Comments: |  |  |  |  | North | Slush | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 19-337309 | 2019-Sep-30, Mon,04:18 | Clear | SMV other | P.D. only | East | Dry | Going ahead | Automobile, station wagon | Ran off road | Lost control |  |
| Comments: |  |  |  |  |  | Dry |  |  |  |  |  |
| 19-272722 | 2019-Aug-09, Fri, 10:30 | Clear | Rear end | P.D. only | East | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |  |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 19-142702 | 2019-Apr-29, Mon,11:31 | Clear | Angle | Non-fatal injury | West | Dry | Going ahead | Passenger van | Other motor vehicle | Disobeyed traffic control |  |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 19-124248 | 2019-Apr-14, Sun, 11:21 | Rain | Angle | Non-fatal injury | North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |  |
| Comments: |  |  |  |  | East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |  |


| 18-252449 | 2018-Aug-10, Fri, 16:00 | Clear | Rear end | Non-fatal injury East |  | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-68978 | 2017-Mar-09, Thu,02:34 | Clear | SMV other | P.D. only | West | Dry | Going ahead | Automobile, station wagon | Ran off road | Lost control |
| Comments: |  |  |  |  |  | Dry |  |  |  |  |
| 16-98242 | 2016-Apr-13, Wed, 17:48 | Clear | Turning movement | Non-fatal | South | Dry | Turning left | Automobile, station wagon | Cyclist | Failed to yield right-ofway |
| Comments: |  |  |  |  | East | Dry | Going ahead | Bicycle | Other motor vehicle | Driving properly |
| 16-78072 | 2016-Mar-24, Thu, 10:31 | Rain | Angle | P.D. only | East | Slush | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | South | Slush | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-51818 | 2016-Feb-24, Wed, 10:35 | Snow | Angle | P.D. only | East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Speed too fast for condition |
| Comments: |  |  |  |  | South | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-266270 | 2016-Oct-01, Sat, 13:20 | Clear | Rear end | P.D. only | South | Wet | Stopped | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | South | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-201087 | 2016-Jul-23, Sat,23:30 | Clear | Rear end | Non-fatal | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Other |
| Comments: |  |  |  |  | East | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-195423 | 2016-Jul-19, Tue, 16:30 | Clear | Sideswipe | Non-fatal | North | Dry | Changing lanes | Automobile, station wagon | Other motor vehicle | Improper lane change |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-156317 | 2016-Jun-07, Tue,09:17 | Clear | Rear end | Non-fatal | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Lost control |
| Comments: |  |  |  |  | South | Dry | Stopped | Pick-up truck | Other motor vehicle | Driving properly |
| 15-339938 | 2015-Dec-16, Wed, 18:36 | Rain | Angle | P.D. only | West | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 14-361263 | 2014-Dec-27, Sat, 13:32 | Clear | Angle | P.D. only | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | North | Dry | Going ahead | Pick-up truck | Other motor vehicle | Driving properly |


| 14-357812 | 2014-Dec-22, Mon,17:10 | Clear | Turning movement | P.D. only | North | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 14-160256 | 2014-Jun-08, Sun, 19:20 | Rain | SMV other | P.D. only | South | Wet | Going ahead | Automobile, station wagon | Ran off road | Lost control |
| Comments: |  |  |  |  |  | Wet |  |  |  |  |
| 14-143495 | 2014-May-23, Fri, 19:43 | Clear | Rear end | P.D. only | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | North | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-237880s | 2013-Aug-28, Wed,08:10 | Clear | Angle | P.D. only | North | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | South |  | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-209744s | 2013-Jul-29, Mon,15:30 | Clear | Rear end | P.D. only | North |  | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  |  |  |  |  |  |  |
| 13-175856 | 2013-Jun-27, Thu, 16:00 | Clear | Rear end | Non-fatal | South | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 12215783 | 2012-Jul-28, Sat, 12:20 | Clear | Rear end | Non-fatal | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | West | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 12169401 | 2012-Jun-14, Thu, 15:18 | Clear | Rear end | Non-fatal | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | East | Dry | Going ahead | Automobile, station wagon |  | Driving properly |
| 11324362 | 2011-Nov-01, Tue, 14:09 | Clear | Angle | P.D. only | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 11218633 | 2011-Jul-24, Sun,11:50 | Clear | Angle | Non-fatal | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1119109s | 2011-Jan-19, Wed, 18:30 | Clear | SMV other | Non-repo | East | Dry | Going ahead | Pick-up truck | Fence/noice barrier | Driving properly |
| Comments: |  |  |  |  |  |  |  |  |  |  |



Collision Details Report
From: January 1, 2010 To: April 30, 2021


| 19-21406 | 2019-Jan-19, Sat,09:22 | Clear | Rear end | P.D. only | West | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | West | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 19-213064 | 2019-Jun-25, Tue,09:23 | Clear | Angle | Non-fatal injury West |  | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-97455 | 2018-Mar-29, Thu, 16:15 | Rain | Turning movement | P.D. only | West | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-83953s | 2018-Mar-17, Sat, 13:30 | Clear | Rear end | P.D. only | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-7717 | 2018-Jan-08, Mon,08:25 | Snow | Rear end | Non-fatal injury North |  | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Lost control |
| Comments: |  |  |  |  | North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-388388 | 2018-Dec-11, Tue,17:53 | Drifting Snow | Turning movement | Non-fatal injury West |  | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-38462 | 2018-Feb-04, Sun,19:09 | Snow | Turning movement | Non-fatal injury West |  | Packed snow | Turning left | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | East | Packed snow | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-343832s | 2018-Oct-31, Wed,09:25 | Rain | Rear end | P.D. only | North | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Other |
| Comments: |  |  |  |  | North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-296616 | 2018-Sep-19, Wed,07:28 | Clear | Angle | Non-fatal injury East |  | Dry | Turning right | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-244113 | 2018-Aug-03, Fri, 13:41 | Clear | Angle | Non-fatal injury West |  | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Other |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 18-223577 | 2018-Jul-17, Tue, 08:55 | Clear | Angle | P.D. only | East | Dry | Turning right | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |


| 18-119928 | 2018-Apr-19, Thu, 15:17 | Clear | Rear end | P.D. only | East | Dry | Changing lanes | Automobile, station wagon | Other motor vehicle | Following too close |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-97739 | 2017-Apr-05, Wed,07:00 | Clear | Rear end | P.D. only | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | West | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-71126 | 2017-Mar-11, Sat,02:03 | Snow | Angle | Non-fatal injury N | North | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | East | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-327236 | 2017-Nov-13, Mon,17:28 | Clear | Turning movement | Non-fatal injury | North | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-262138 | 2017-Sep-10, Sun, 11:34 | Clear | Turning movement | P.D. only | North | Dry | Turning left | Passenger van | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-233082s | 2017-Apr-11, Tue,09:23 | Clear | Rear end | P.D. only | West | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | West | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-126090 | 2017-Apr-30, Sun,08:33 | Clear | Angle | P.D. only | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  |  | West | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-99372 | 2016-Apr-14, Thu, 18:15 | Clear | Rear end | P.D. only | East | Dry | Going ahead | Passenger van | Other motor vehicle | Other |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-72299 | 2016-Mar-17, Thu, 17:22 | Clear | Rear end | P.D. only | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-335942s | 2016-Dec-16, Fri, 14:50 | Clear | Rear end | P.D. only | West | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | West | Wet | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-319659 | 2016-Nov-28, Mon,18:11 | Clear | Turning movement | P.D. only | East | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |






| 1283978 | 2012-Mar-23, Fri, 12:40 | Clear | Angle | Non-fatal injury South | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1265587 | 2012-Mar-03, Sat, 12:00 | Clear | Rear end | Non-fatal injury North | Dry | Turning right | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | North | Dry | Turning right | Automobile, station wagon | Other motor vehicle |  |
| 12272588 | 2012-Sep-22, Sat, 16:46 | Clear | Rear end | P.D. only West | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 12218226s | 2012-Jul-29, Sun,21:51 | Clear | Rear end | Non-reportable North | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | North |  |  |  |  |  |
| 12199688s | 2012-Jul-12, Thu, 16:45 | Clear | Rear end | Non-reportable East | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle |  |
| Comments: |  |  |  | East |  |  |  |  |  |
| 12185469 | 2012-Jun-29, Fri, 18:02 | Clear | Angle | P.D. only South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 12132950 | 2012-May-10, Thu, 17:59 | Clear | Angle | P.D. only South | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 12112334s | 2012-Apr-20, Fri, 12:00 | Clear | Rear end | Non-reportable West | Dry | Going ahead |  | Other motor vehicle | Following too close |
| Comments: |  |  |  | West | Dry | Slowing or stopping | Passenger van | Other motor vehicle | Driving properly |
| 1172049s | 2011-Mar-11, Fri, 15:15 | Rain | Sideswipe | Non-reportable East | Wet | Going ahead | Truck - closed | Other motor vehicle | Driving properly |
| Comments: |  |  |  | East | Wet | Changing lanes | Automobile, station wagon | Other motor vehicle | Improper lane change |
| 1152750 | 2011-Feb-21, Mon,03:50 | Snow | SMV other | P.D. only South | Packed snow | Going ahead | Automobile, station wagon | Skidding/sliding | Lost control |
| Comments: |  |  |  |  | Packed snow |  |  |  |  |
| 1143797s | 2011-Feb-12, Sat,18:15 | Clear | Angle | Non-reportable South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | West | Dry | Turning left | Passenger van | Other motor vehicle | Improper turn |
| 11376042s | 2011-Dec-22, Thu,08:30 | Clear | Rear end | Non-reportable West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | West |  |  |  |  |  |


| 11367743 | 2011-Dec-12, Mon,18:30 | Rain | Rear end | Non-fatal injury East | Wet | Stopped | Passenger van | Other motor vehicle | Driving properly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  | East | Wet | Changing lanes | Automobile, station wagon | Other motor vehicle | Following too close |
| 1135872 | 2011-Feb-05, Sat,09:08 | Clear | Angle | Non-fatal injury West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  | South | Dry | Turning left | Pick-up truck | Other motor vehicle | Driving properly |
| 11251033 | 2011-Aug-23, Tue,07:00 | Clear | Angle | P.D. only South | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 11250598 | 2011-Aug-22, Mon,18:19 | Clear | Angle | Non-fatal injury West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | North | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| 11245316 | 2011-Aug-17, Wed, 18:30 | Rain | Rear end | P.D. only North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  | North | Wet | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 11131165 | 2011-May-05, Thu, 14:40 | Clear | Angle | Non-fatal injury East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  | South | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1082267 | 2010-Mar-29, Mon, 14:20 | Clear | Angle | P.D. only North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1079747 | 2010-Mar-26, Fri, 16:58 | Clear | Turning movement | Non-fatal injury East | Dry | Turning left | Automobile, station wagon |  | Improper turn |
| Comments: |  |  |  | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1076535 | 2010-Mar-23, Tue, 12:50 | Rain | Angle | Non-fatal injury West | Wet | Going ahead | Automobile, station wagon |  | Disobeyed traffic control |
| Comments: |  |  |  | South | Wet | Going ahead | Automobile, station wagon |  | Driving properly |
| 10338039 | 2010-Dec-08, Wed, 14:54 | Clear | Angle | Non-fatal injury East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  | South | Dry | Going ahead | Automobile, station wagon |  | Driving properly |
| 10299909 | 2010-Oct-29, Fri, 17:28 | Clear | Approaching | Non-fatal injury East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |


| 10235626s | 2010-Aug-26, Thu, 11:00 | Clear | Rear end | Non-repo | Nest | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: | West |  |  |  |  |  |  |  |  |  |
| 1021344 | 2010-Jan-23, Sat,20:48 | Clear | Turning movement | Non-fatal injury South |  | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  | North |  | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 10171106 | 2010-Jun-25, Fri, 12:55 | Clear | Turning movement | Non-fatal injury South |  | Dry | Turning left | Pick-up truck | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  | North |  | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 10169835s | 2010-Jun-24, Thu,09:00 | Rain | Rear end | Non-reportable West |  | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | West | Wet | Stopped | Truck - closed | Other motor vehicle |  |
| 10168679 | 2010-Jun-23, Wed,07:20 | Clear | Rear end | P.D. only | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  | South |  |  | Stopped | Pick-up truck | Other motor vehicle | Driving properly |
| 10134707s | 2010-May-20, Thu, 14:00 | Clear | Rear end | Non-repo | East | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 10111376 | 2010-Apr-27, Tue,07:37 | Clear | Angle | P.D. only | West | Dry |  |  |  | Improper lane change |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |

Collision Details Report


| 17-305380 | 2017-Oct-22, Sun,23:00 | Clear | SMV other | P.D. only North | Dry | Going ahead | Automobile, station wagon | Animal - wild | Driving properly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: deer |  |  |  |  | Dry |  |  |  |  |
| 15-55121 | 2015-Feb-25, Wed,11:51 | Drifting Snow | SMV other | P.D. only North | Loose snow | Going ahead | Automobile, station wagon | Steel guide rail | Lost control |
| Comments: |  |  |  |  | Loose snow |  |  |  |  |
| $15-53740$ | 2015-Feb-25, Wed,02:28 | Clear | SMV other | P.D. only North | Dry | Going ahead | Automobile, station wagon | Other | Lost control |
| Comments: |  |  |  |  |  |  |  |  |  |
| 15-53498 | 2015-Feb-24, Tue, 18:30 | Drifting Snow | Approaching | Non-fatal injury South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Lost control |
| Comments: |  |  |  | North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 15-2402 | 2015-Jan-03, Sat, 18:43 | Clear | SMV other | P.D. only South | Wet | Going ahead | Automobile, station wagon | Skidding/sliding | Speed too fast for condition |
| Comments: |  |  |  |  | Wet |  |  |  |  |
| 15-200387 | 2015-Jul-19, Sun,12:51 | Clear | Turning movement | Non-fatal injury North | Dry | Overtaking | Passenger van | Other motor vehicle | Lost control |
| Comments: |  |  |  | North | Dry | Turning right | Automobile, station wagon | Other motor vehicle | Driving properly |
| 14-319690 | 2014-Nov-09, Sun,17:00 | Clear | SMV other | P.D. only North | Dry | Going ahead | Automobile, station wagon | Animal - wild | Driving properly |
| Comments: |  |  |  |  | Dry |  |  |  |  |
| 14-236737 | 2014-Aug-18, Mon,16:07 | Clear | SMV other | Non-fatal injury South | Dry | Going ahead | Automobile, station wagon | Ran off road | Lost control |
| Comments: |  |  |  |  |  |  |  |  |  |
| 12229163 | 2012-Aug-10, Fri,09:26 | Rain | Rear end | P.D. only North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  | North |  | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| 12130288 | 2012-May-08, Tue,03:00 | Rain | SMV other | Non-fatal injury South | Wet | Going ahead | Automobile, station wagon | Skidding/sliding | Lost control |
| Comments: |  |  |  |  |  |  |  |  |  |
| 11212900 | 2011-Jul-19, Tue, 05:12 | Clear | SMV other | Non-fatal injury West | Dry | Going ahead | Automobile, station wagon | Pole (utility, power) | Lost control |
| Comments: |  |  |  |  | Dry |  |  |  |  |
| 1092534s | 2010-Apr-08, Thu, 18:30 | Clear | Rear end | Non-reportable North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | North | Wet | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |


| 1068175 | 2010-Mar-14, Sun,08:19 | Rain | SMV other | P.D. only | North | Wet | Going ahead | Passenger van | Ditch | Lost control |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  |  | Wet |  |  |  |  |
| 10316872s | 2010-Nov-13, Sat, 14:30 | Clear | Rear end | Non-reportable | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | South | Dry | Turning right | Automobile, station wagon | Other motor vehicle | Driving properly |
| 10299037 | 2010-Oct-28, Thu, 19:26 | Clear | Rear end | P.D. only | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | North |  | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |

Collision Details Report

From: January 1, 2010 To: April 30, 2021


| 17-42527s | 2017-Feb-10, Fri, 15:30 | Snow | Angle | P.D. only | East | Slush | Turning right | Automobile, station wagon | Other motor vehicle | Improper turn |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | North | Slush | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-359530s | 2017-Dec-15, Fri,16:55 | Snow | Angle | P.D. only | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | East | Wet | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-357142s | 2017-Dec-13, Wed,08:00 | Clear | Rear end | P.D. only | South | Dry | Stopped | Delivery van | Other motor vehicle | Other |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-228220 | 2017-Aug-06, Sun, 14:50 | Clear | Rear end | P.D. only | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | West | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-173878s | 2017-Jun-14, Wed,17:15 | Clear | Rear end | P.D. only | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | North | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-51691 | 2016-Feb-24, Wed,08:07 | Snow | Sideswipe | P.D. only | South | Slush | Going ahead | Pick-up truck | Other motor vehicle | Lost control |
| Comments: |  |  |  |  | North | Slush | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-336351 | 2016-Dec-17, Sat,04:39 | Snow | SMV other | P.D. only | West | Loose snow | Going ahead | Automobile, station wagon | Tree, shrub, stump | Lost control |
| Comments: |  |  |  |  |  | Loose snow |  |  |  |  |
| 16-324942 | 2016-Dec-04, Sun, 14:45 | Clear | Turning movement | P.D. only | North | Dry | Changing lanes | Automobile, station wagon | Other motor vehicle | Improper lane change |
| Comments: |  |  |  |  | North | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-304437 | 2016-Nov-11, Fri,20:05 | Clear | SMV other | P.D. only | South | Dry | Going ahead | Automobile, station wagon | Animal - wild | Driving properly |
| Comments: |  |  |  |  |  | Dry |  |  |  |  |
| 15-312148 | 2015-Nov-14, Sat, 18:06 | Clear | SMV other | P.D. only | North | Dry | Going ahead | Automobile, station wagon | Animal - wild | Driving properly |
| Comments: |  |  |  |  |  | Dry |  |  |  |  |
| 15-251549 | 2015-Sep-09, Wed,07:55 | Rain | SMV other | P.D. only | South | Wet | Going ahead | Automobile, station wagon | Ditch | Following too close |
| Comments: |  |  |  |  |  | Wet |  |  |  |  |
| 15-112095 | 2015-Apr-25, Sat,06:35 | Clear | Sideswipe | Non-fatal | East | Dry | Going ahead | Automobile, station wagon | Cyclist | Speed too fast for condition |
| Comments: |  |  |  |  | East | Dry | Going ahead | Bicycle | Other motor vehicle | Driving properly |


| 14-57152 | 2014-Feb-27, Thu,09:50 | Snow | Rear end | Non-fatal injury South | Loose snow | Going ahead | Pick-up truck | Other motor vehicle | Driving properly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  | South | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 14-323525 | 2014-Nov-13, Thu,20:00 | Clear | SMV other | P.D. only North | Dry | Going ahead | Pick-up truck | Animal - wild | Driving properly |
| Comments: |  |  |  |  | Dry |  |  |  |  |
| 14-266228s | 2014-Sep-15, Mon,15:30 | Clear | Rear end | P.D. only North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  | North |  | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 14-228410 | 2014-Aug-10, Sun, 13:15 | Clear | SMV other | Non-fatal injury West | Dry | Turning left | Automobile, station wagon | Ran off road | Lost control |
| Comments: |  |  |  |  | Dry |  |  |  |  |
| 14-131403 | 2014-May-11, Sun,17:11 | Clear | Sideswipe | Non-fatal injury West | Dry | Overtaking | Automobile, station wagon | Cyclist | Improper passing |
| Comments: |  |  |  | West |  | Going ahead | Bicycle | Other motor vehicle | Driving properly |
| 14-054214s | 2014-Feb-24, Mon,17:08 | Clear | Rear end | P.D. only South | Ice | Going ahead |  | Other motor vehicle | Driving properly |
| Comments: |  |  |  | South |  | Stopped |  | Other motor vehicle |  |
| 13-262107s | 2013-Sep-22, Sun,15:50 | Clear | Angle | P.D. only West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | South |  | Turning left | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| 13-244964s | 2013-Sep-04, Wed,08:10 | Clear | Rear end | P.D. only West | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  | West |  | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-175158 | 2013-Jun-26, Wed,23:20 | Clear | Approaching | Non-fatal injury West | Dry | Going ahead | Automobile, station wagon | Ran off road | Speed too fast for condition |
| Comments: |  |  |  |  | Dry |  |  |  |  |
| 1230886 | 2012-Jan-31, Tue,08:39 | Clear | Angle | Non-fatal injury West | Wet | Turning left |  | Other motor vehicle | Improper turn |
| Comments: |  |  |  | South | Wet | Going ahead |  | Other motor vehicle | Driving properly |
| 12262425 | 2012-Sep-12, Wed, 14:54 | Clear | SMV other | Non-fatal injury East | Dry | Other | Motorcycle | Ran off road | Exceeding speed limit |
| Comments: |  |  |  |  | Dry |  |  |  |  |
| 1148957s | 2011-Feb-17, Thu,07:45 | Clear | Rear end | Non-reportable South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  | South |  |  |  |  |  |


| 11315914 | 2011-Oct-24, Mon,11:57 | Clear | Angle | P.D. only East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  | North | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Improper lane change |
| 10323495 | 2010-Nov-23, Tue,07:20 | Other | Angle | Non-fatal injury South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Lost control |
| Comments: |  |  |  | West | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Driving properly |
| 10319017s | 2010-Nov-17, Wed,16:45 | Rain | Rear end | Non-reportable South | Wet | Slowing or stopping | Automobile, station wagon |  | Driving properly |
| Comments: |  |  |  | South |  |  |  |  |  |
| 10232645s | 2010-Aug-23, Mon,18:14 | Clear | Rear end | Non-reportable East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  | East | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |

Collision Details Report


Collision Details Report

From: January 1, 2010 To: April 30, 2021


| 1050160 | 2010-Feb-23, Tue, 10:28 | Clear | Sideswipe | P.D. only | North | Dry | Turning right | Truck-other | Other motor vehicle | Driving properly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | North | Dry | Turning right | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1083447 | 2010-Mar-30, Tue,18:00 | Clear | Turning movement | P.D. only | West | Dry | Turning left |  | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1093258 | 2010-Apr-09, Fri, 13:06 | Clear | Angle | Non-fatal injury | West | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 11152880 | 2011-May-25, Wed, 10:40 | Clear | Rear end | P.D. only | North | Dry | Slowing or stopping | Ambulance | Other motor vehicle | Lost control |
| Comments: |  |  |  |  | North | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 11209089 | 2011-Jul-15, Fri, 17:30 | Clear | Rear end | Non-fatal injury | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 11228721s | 2011-Aug-02, Tue,14:00 | Clear | Rear end | Non-reportable | West | Dry | Stopped |  | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | West | Dry | Slowing or stopping |  | Other motor vehicle |  |
| 11292039 | 2011-Sep-30, Fri,20:25 | Rain | Angle | P.D. only | North | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Speed too fast for condition |
| 11293612s | 2011-Oct-02, Sun,13:00 | Rain | Rear end | Non-reportable |  | Wet | Going ahead |  | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | East |  |  |  |  |  |
| 11294330s | 2011-Oct-02, Sun,13:00 | Clear | Rear end | Non-reportable |  | Dry | Stopped |  | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | East |  |  |  |  |  |
| 11324037 | 2011-Nov-01, Tue,09:25 |  | Turning movement | P.D. only | West | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 11333548s | 2011-Nov-10, Thu,08:15 | Clear | Turning movement | Non-reportable | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | West | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |


| 11353780s | 2011-Nov-30, Wed, 15:35 | Clear | Angle | Non-reportable North | Dry | Turning right | Automobile, station wagon | Other motor vehicle | Driving properly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle |  |
| 1146278 | 2011-Feb-15, Tue,09:04 | Clear | Angle | Non-fatal injury South | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1150753 | 2011-Feb-19, Sat,03:00 | Strong wind | Approaching | Non-fatal injury East | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Speed too fast for condition |
| Comments: |  |  |  | West | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1198861s | 2011-Apr-06, Wed,08:45 | Clear | Rear end | Non-reportable South | Dry | Stopped |  | Other motor vehicle | Driving properly |
| Comments: |  |  |  | South | Dry | Slowing or stopping |  | Other motor vehicle | Following too close |
| 12105255s | 2012-Apr-13, Fri, 15:45 | Clear | Rear end | Non-reportable North | Dry | Going ahead |  | Other motor vehicle | Following too close |
| Comments: |  |  |  | North | Dry | Slowing or stopping |  | Other motor vehicle | Driving properly |
| 12132248s | 2012-May-09, Wed, 17:30 | Clear | Rear end | Non-reportable East | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle |  |
| Comments: |  |  |  | East |  |  |  |  |  |
| 1218764 | 2012-Jan-19, Thu, 11:18 | Snow | Angle | P.D. only North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Disobeyed traffic control |
| Comments: |  |  |  | East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1219151s | 2012-Jan-19, Thu, 14:41 | Clear | Rear end | Non-reportable North | Dry | Stopped |  | Other motor vehicle | Driving properly |
| Comments: |  |  |  | North | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Speed too fast for condition |
| 12230538 | 2012-Aug-11, Sat, 17:09 | Rain | Rear end | P.D. only West | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  | West | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 12262059 | 2012-Sep-12, Wed,08:48 | Clear | Rear end | P.D. only West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Improper lane change |
| Comments: |  |  |  | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 12267825s | 2012-Sep-17, Mon,17:20 | Clear | Rear end | Non-reportable East | Dry | Going ahead | Truck - closed | Other motor vehicle | Driving properly |
| Comments: |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle |  |


| 12273498 | 2012-Sep-23, Sun,16:08 | Rain | Rear end | P.D. only | East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Speed too fast for condition |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | East | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 1230952 | 2012-Jan-31, Tue,09:00 | Clear | SMV other | P.D. only | North | Wet | Going ahead | Truck - dump | Other | Driving properly |
| Comments: |  |  |  |  |  | Wet |  |  |  |  |
| 12329767 | 2012-Nov-21, Wed, 18:35 | Fog, mist, smoke, dust | Angle | P.D. only | West | Dry | Making "U" turn | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-035635s | 2013-Feb-06, Wed, 12:30 |  | Approaching | P.D. only | East |  | Going ahead | Automobile, station wagon | Other motor vehicle | Improper lane change |
| Comments: |  |  |  |  | East |  | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-124159 | 2013-May-07, Tue, 17:31 | Clear | Rear end | Non-fatal injury N | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | North | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-126199 | 2013-May-09, Thu,16:58 | Clear | Angle | P.D. only | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | North | Dry | Changing lanes | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-173347 | 2013-Jun-25, Tue,09:00 | Rain | Sideswipe | P.D. only | South | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Improper lane change |
| Comments: |  |  |  |  | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-186805 | 2013-Jul-08, Mon,09:50 | Clear | Rear end | P.D. only | South | Dry | Reversing | Automobile, station wagon | Other motor vehicle | Other |
| Comments: |  |  |  |  | South | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-195129 | 2013-Jul-16, Tue,09:45 | Clear | SMV other | Non-fatal injury | West | Dry | Going ahead | Automobile, station wagon | Ran off road | Lost control |
| Comments: |  |  |  |  |  | Dry |  |  |  |  |
| 13-34275 | 2013-Feb-05, Tue, 11:59 | Clear | Rear end | P.D. only | West | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Exceeding speed limit |
| Comments: |  |  |  |  | West | Wet | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-39259 | 2013-Feb-11, Mon,06:25 | Freezing Rain | Approaching | Non-fatal injury | East | Ice | Going ahead | Automobile, station wagon | Other motor vehicle | Lost control |
| Comments: |  |  |  |  | West | Ice | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-85138 | 2013-Mar-29, Fri, 13:48 | Clear | SMV other | P.D. only | East | Dry | Turning right | Automobile, station wagon | Curb | Lost control |


| 14-114315s | 2014-Apr-24, Thu, 19:00 | Clear | Rear end | P.D. only | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | East |  | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Following too close |
| 14-131636 | 2014-May-11, Sun,22:51 | Clear | Rear end | P.D. only | East | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Other |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 14-263494 | 2014-Sep-12, Fri, 17:18 | Clear | Rear end | Non-fatal injury | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 14-328945 | 2014-Nov-20, Thu,08:19 | Clear | Rear end | P.D. only | East | Wet | Going ahead | School bus | Other motor vehicle | Speed too fast for condition |
| Comments: |  |  |  |  | East | Loose snow | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 15-108188 | 2015-Apr-21, Tue,09:30 | Rain | Rear end | Non-fatal injury | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Other |
| Comments: |  |  |  |  | South | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 15-11183 | 2015-Jan-12, Mon, 17:59 | Clear | Turning movement | Non-fatal injury | South | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | East | Wet | Going ahead | Automobile, station wagon | Steel guide rail | Driving properly |
| 15-159536 | 2015-Jun-07, Sun,04:00 | Clear | Rear end | P.D. only | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 15-162735 | 2015-Jun-11, Thu, 17:21 | Clear | Rear end | Non-fatal injury | North | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Other |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 15-167356 | 2015-Jun-17, Wed, 12:15 | Clear | Angle | P.D. only | West | Dry | Going ahead | Bicycle | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | North | Dry | Slowing or stopping | Automobile, station wagon | Cyclist | Driving properly |
| 15-175421 | 2015-Jun-25, Thu,13:15 | Clear | Other | P.D. only | South | Dry | Reversing | Pick-up truck | Other motor vehicle | Improper lane change |
| Comments: |  |  |  |  | South | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 15-193996 | 2015-Jul-13, Mon,18:23 | Clear | Turning movement | P.D. only | West | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | East | Dry | Going ahead | Pick-up truck | Other motor vehicle | Driving properly |



| 16-294863 | 2016-Nov-01, Tue,17:06 | Clear | Rear end | P.D. only | East | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Improper lane change |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | East | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-320294 | 2016-Nov-29, Tue,11:40 | Clear | Rear end | Non-fatal | West | Dry | Overtaking | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-330031 | 2016-Dec-09, Fri, 17:33 | Clear | Turning movement | P.D. only | North | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-338642 | 2016-Dec-20, Tue,08:09 | Clear | Turning movement | Non-fata | South | Dry | Turning left | Delivery van | Other motor vehicle | Failed to yield right-ofway |
| Comments: |  |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-37605 | 2016-Feb-08, Mon,18:03 | Clear | Rear end | P.D. only | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Lost control |
| Comments: |  |  |  |  | East | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-43037 | 2016-Feb-14, Sun,16:37 | Clear | Turning movement | P.D. only | South | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | East | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-135145 | 2017-May-09, Tue,08:59 | Clear | Rear end | Non-fatal | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | South | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-162790 | 2017-Jun-05, Mon,07:36 | Clear | Turning movement | P.D. only | East | Dry | Making "U" turn | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | West | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-18767 | 2017-Jan-19, Thu, 17:29 | Clear | Turning movement | Non-fatal | South | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | North | Dry | Turning left | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-224340 | 2017-Aug-02, Wed, 17:40 | Rain | Turning movement | Non-fatal | North | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 17-23614 | 2017-Jan-24, Tue, 14:52 | Rain | Turning movement | P.D. only | North | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |




| 20-71724 | 2020-Feb-26, Wed, 17:33 | Snow | Rear end | P.D. only | East | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Lost control |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | East | Loose snow | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |
| 21-21826 | 2021-Jan-19, Tue,08:53 | Clear | Turning movement | P.D. only | West | Wet | Turning left | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | East | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |

Collision Details Report


| 16-84731 | 2016-Mar-31, Thu,09:23 | Rain | Rear end | P.D. only | South | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Speed too fast for condition |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  | South | Wet | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 16-103382 | 2016-Apr-18, Mon,15:49 | Clear | Sideswipe | P.D. only | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Improper turn |
| Comments: |  |  |  |  | North | Dry | Stopped | Pick-up truck | Other motor vehicle | Driving properly |
| 14-57029 | 2014-Feb-27, Thu,09:58 | Strong wind | Rear end | P.D. only | South | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Speed too fast for condition |
| Comments: |  |  |  |  | South | Loose snow | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-57863 | 2013-Mar-01, Fri, 16:45 | Clear | Rear end | Non-fatal injury | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |
| Comments: |  |  |  |  | North | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |
| 13-39263 | 2013-Feb-11, Mon,06:46 | Rain | SMV other | P.D. only | South | Wet | Going ahead | Automobile, station wagon | Ran off road | Lost control |
| Comments: |  |  |  |  |  |  |  |  |  |  |
| 1052586 | 2010-Feb-25, Thu, 16:52 | Drifting Snow | SMV other | Non-fatal injury | South | Loose snow | Going ahead | Automobile, station wagon | Ditch | Lost control |
| Comments: |  |  |  |  |  |  |  |  |  |  |
| 10223938 | 2010-Aug-15, Sun, 12:30 | Clear | Sideswipe | P.D. only | South | Dry | Overtaking | Automobile, station wagon | Other motor vehicle | Improper passing |
| Comments: |  |  |  |  | South |  | Going ahead | Passenger van | Other motor vehicle | Driving properly |

Collision Details Report

| York Region |  |  |  |  |  |  |  | From: January 1, 2010 |  | To: April 30, 2021 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location $\qquad$ Warden Avenue btwn Majo Traffic Control.... |  |  |  |  |  |  |  | Municipality........ Markham <br> Total Collisions.... 13 |  |  |  |
| Collision ID D | Date/Day/Time | Environment | Impact Type | Classification | Direction | Surface Cond'n | Vehicle Manoeuver | Vehicle type | First Event | Driver Action | No. Ped |
| 19-422602 | 2019-Dec-11, Wed,07:40 | Clear | Rear end | P.D. only | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |  |
| Comments: |  |  |  |  | South | Dry | Slowing or stopping | School bus | Other motor vehicle | Driving properly |  |
| 18-258603 | 2018-Aug-15, Wed, 17:37 |  | Rear end | Non-fatal injury | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |  |
| Comments: |  |  |  |  | North | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 18-224831s | 2018-Jul-18, Wed,08:50 | Clear | Rear end | P.D. only | South | Dry | Slowing or stopping | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| Comments: |  |  |  |  | South | Dry | Slowing or stopping | Passenger van | Other motor vehicle | Driving properly |  |
| 18-130586 | 2018-Apr-28, Sat, 18:08 | Clear | Rear end | P.D. only | North | Dry | Making "U" turn | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| Comments: |  |  |  |  | North | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 16-290202 | 2016-Oct-27, Thu,14:30 | Rain | SMV other | P.D. only | North | Wet | Going ahead | Automobile, station wagon | Animal - wild | Lost control |  |
| Comments: |  |  |  |  |  | Wet |  |  |  |  |  |
| 16-124107 | 2016-May-09, Mon,08:00 | Clear | Rear end | P.D. only | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle |  |  |
| Comments: |  |  |  |  | South | Dry | Stopped | Automobile, station wagon | Other motor vehicle | Driving properly |  |
| 16-113568 | 2016-Apr-28, Thu,08:05 | Clear | Rear end | P.D. only | South | Dry | Going ahead | Automobile, station wagon | Other motor vehicle | Following too close |  |
| Comments: |  |  |  |  | South |  | Slowing or stopping | Pick-up truck | Other motor vehicle | Driving properly |  |
| 15-43268 | 2015-Feb-14, Sat, 20:54 | Drifting Snow | SMV other | P.D. only | North | Loose snow | Going ahead | Automobile, station wagon | Ran off road | Speed too fast for condition |  |
| Comments: |  |  |  |  |  | Loose snow |  |  |  |  |  |
| 13-51084 | 2013-Feb-22, Fri, 18:53 | Snow | Approaching | P.D. only | South | Slush | Going ahead | Automobile, station wagon | Skidding/sliding | Lost control |  |
| Comments: |  |  |  |  | North |  | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |  |


| 13-23905 | 2013-Jan-25, Fri, 14:35 | Rain | SMV other | P.D. only | South | Wet | Going ahead | Automobile, station wagon | Ran off road | Driving properly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Comments: |  |  |  |  |  |  |  |  |  |  |
| 12345012 | 2012-Dec-08, Sat,02:54 | Clear | SMV unattendedP.D. only vehicle |  | North | Dry | Going ahead | Automobile, station wagon | Animal - wild | Driving properly |
| Comments: |  |  |  |  |  | Dry |  |  |  |  |
| 10270263s | 2010-Sep-28, Tue, 11:15 | Rain | Sideswipe | Non-repor | North | Wet | Going ahead | Automobile, station wagon | Other motor vehicle | Driving properly |
| Comments: |  |  |  |  | North |  |  |  |  |  |
| 10133675s | 2010-May-17, Mon,23:00 | Clear | SMV unatten vehicle | dNon-repor | South | Dry | Going ahead | Automobile, station wagon | Animal - wild | Driving properly |
| Comments: |  |  |  |  |  |  |  |  |  |  | Burnside

[The Difference is our People]

## Appendix E

## 2021 Existing Conditions Synchro Reports



| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\stackrel{1}{ }$ | \% | $\hat{\square}$ | \% | $\uparrow$ | 「 | ${ }^{7}$ | $\uparrow$ | F |
| Trafic Volume (vph) | 19 | 293 | 137 | 681 | 79 | 149 | 11 | 6 | 830 | 80 |
| Future Volume (vph) | 19 | 293 | 137 | 681 | 79 | 149 | 11 | 6 | 830 | 80 |
| Lane Group Flow (vph) | 22 | 498 | 159 | 808 | 92 | 173 | 13 | 7 | 965 | 93 |
| Turn Type | Perm | NA | Perm | NA | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  | 8 |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 8 |  | 6 |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 10.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| Minimum Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (\%) | 52.2\% | 52.2\% | 52.2\% | 52.2\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% |
| Yellow Time (s) | 4.5 | 4.5 | 4.5 | 4.5 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 2.5 | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | Max | Max | Max | Max | Max | Max |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.21 | 0.68 | 0.60 | 1.03 | 0.83 | 0.25 | 0.02 | 0.02 | 1.37 | 0.15 |
| Control Delay | 18.5 | 20.6 | 27.4 | 62.0 | 75.8 | 15.8 | 0.1 | 13.5 | 200.6 | 6.9 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 18.5 | 20.6 | 27.4 | 62.0 | 75.8 | 15.8 | 0.1 | 13.5 | 200.6 | 6.9 |
| Queue Length 50th (m) | 1.7 | 45.2 | 14.8 | ~104.0 | 10.1 | 14.5 | 0.0 | 0.5 | ~163.4 | 2.2 |
| Queue Length 95th (m) | 6.5 | 70.8 | \#33.0 | \#160.4 | \#32.9 | 25.8 | 0.0 | 2.7 | \#213.2 | 9.4 |
| Internal Link Dist (m) |  | 180.4 |  | 543.1 |  | 1650.0 |  |  | 238.4 |  |
| Turn Bay Length (m) | 70.0 |  | 80.0 |  | 125.0 |  | 30.0 | 100.0 |  | 30.0 |
| Base Capacity (vph) | 106 | 729 | 263 | 786 | 111 | 682 | 650 | 464 | 702 | 632 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.21 | 0.68 | 0.60 | 1.03 | 0.83 | 0.25 | 0.02 | 0.02 | 1.37 | 0.15 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 67 |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 67 |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 100 |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord <br>  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |


| Splits and Phases: 1: Warden Avenue \& Elgin Mills Road East |  |  |
| :---: | :---: | :---: |
| ${ }_{*}{ }^{\circ}$ | $\rightarrow{ }^{4}$ |  |
| 32 s | 35 s |  |
| ${ }^{4} 06$ | - $\square_{08}$ |  |
| 32 s | 35 s |  |
| AMandPM_Existing_20220426.syn Synchro 11 Report |  |  |
| R.J. Burnside \& Associates |  | 05/20/2022-Page 1 |

HCM Signalized Intersection Capacity Analysis
1: Warden Avenue \& Elgin Mills Road East

c Critical Lane Group

2：Warden Avenue \＆Major Mackenzie Drive

|  | $\Rightarrow$ |  |  |  |  |  | 4 | 4 | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | ¢ $\uparrow$ | F | \％ | 个 $\uparrow$ | 7 | \％ | 个个 | F | \％ | $\uparrow$ | 「 |
| Traffic Volume（vph） | 1 | 613 | 133 | 327 | 1844 | 19 | 62 | 146 | 61 | 30 | 994 | 88 |
| Future Volume（vph） | 1 | 613 | 133 | 327 | 1844 | 19 | 62 | 146 | 61 | 30 | 994 | 88 |
| Lane Group Flow（vph） | 1 | 659 | 143 | 352 | 1983 | 20 | 67 | 157 | 66 | 32 | 1069 | 95 |
| Turn Type | Perm | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 6 |  | 5 | 2 |  | ， | 8 |  |  | 4 |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  |  |
| Detector Phase | 6 | 6 | 6 | 5 | 2 | 2 | 3 | 8 | 8 | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 34.0 | 34.0 | 34.0 | 7.0 | 34.0 | 34.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Minimum Split（s） | 41.5 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 41.5 | 41.5 | 41.5 |
| Total Split（s） | 43.0 | 43.0 | 43.0 | 25.0 | 68.0 | 68.0 | 12.0 | 62.0 | 62.0 | 50.0 | 50.0 | 50.0 |
| Total Split（\％） | 33．1\％ | 33．1\％ | 33．1\％ | 19．2\％ | 52．3\％ | 52．3\％ | 9．2\％ | 47．7\％ | 47．7\％ | 38．5\％ | 38．5\％ | 38．5\％ |
| Yellow Time（s） | 5.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |
| All－Red Time（s） | 2.5 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | －3．0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 7.5 | 7.5 | 7.5 | 4.0 | 4.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag | Lag | Lag | Lag | Lead |  |  | Lead |  |  | Lag | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes |  |  | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | Max | Max | Max | None | Max | Max | None | None | None | None | None | None |
| $\mathrm{v} / \mathrm{R}$ Ratio | 0.02 | 0.66 | 0.26 | 0.85 | 1.10 | 0.02 | 0.41 | 0.11 | 0.11 | 0.08 | 1.68 | 0.15 |
| Control Delay | 36.0 | 44.5 | 6.9 | 40.4 | 85.7 | 0.1 | 28.6 | 23.5 | 5.0 | 31.2 | 344.3 | 5.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 36.0 | 44.5 | 6.9 | 40.4 | 85.7 | 0.1 | 28.6 | 23.5 | 5.0 | 31.2 | 344.3 | 5.2 |
| Queue Length 50th（m） | 0.2 | 80.1 | 0.0 | 55.6 | $\sim 308.2$ | 0.0 | 9.7 | 12.6 | 0.0 | 5.7 | $\sim 403.5$ | 0.0 |
| Queue Length 95th（m） | 1.8 | 101.2 | 15.6 | \＃96．6 | \＃349．6 | 0.0 | 18.6 | 19.8 | 7.8 | 13.4 | \＃482．2 | 10.0 |
| Internal Link Dist（ m ） |  | 203.6 |  |  | 501.9 |  |  | 120.0 |  |  | 316.9 |  |
| Turn Bay Length（ $m$ ） | 58.0 |  | 90.0 | 138.0 |  | 111.0 | 48.0 |  | 32.0 | 146.0 |  | 80.0 |
| Base Capacity（vph） | 60 | 1000 | 557 | 432 | 1803 | 813 | 167 | 1503 | 633 | 380 | 635 | 615 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |  |
| Reduced v／c Ratio | 0.02 | 0.66 | 0.26 | 0.81 | 1.10 | 0.02 | 0.40 | 0.10 | 0.10 | 0.08 | 1.68 | 0.15 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 130 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 127.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 145 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Semi Act－Uncoord <br>  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |


| Splits and Phases：2：Warden Avenue \＆Major Mackenzie Drive |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| $4 \square_{02}$ |  | 403 | $\stackrel{1}{*}$ |  |
| 58 s |  | 12 s | 50 s |  |
| ${ }_{605}$ | $\rightarrow 6$ | ${ }^{4} 0$ |  |  |
| 125 s | 43 s | 62 s |  |  |
| AMandPM＿Existing＿20220426．syn <br> R．J．Burnside \＆Associates |  |  |  | Synchro 11 Report 05／20／2022－Page 3 |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％ | ¢ $\uparrow$ | F | \％ | 个个 | 「 | ${ }^{7}$ | 个 $\uparrow$ | 「 | \％ | $\uparrow$ | 7 |
| Traffic Volume（vph） | 1 | 613 | 133 | 327 | 1844 | 19 | 62 | 146 | 61 | 30 | 994 | 88 |
| Future Volume（vph） | 1 | 613 | 133 | 327 | 1844 | 19 | 62 | 146 | 61 | 30 | 994 | 88 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 7.5 | 7.5 | 7.5 | 4.0 | 4.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.0 |


|  | 7.5 | 7.5 | 7.5 | 4.0 | 4.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Total Lost time（s） | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Lane Uti．Factor | 1.00 | 1.00 | 0.8 | 1.00 | 1.00 | 0 | 1.0 | 1.00 | 0 | 1.0 | 10 |


| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |


| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 1825 | 3444 | 1570 | 1772 | 3614 | 1633 | 1738 | 3510 | 1384 | 1659 | 1902 | 163 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Satd．Flow（prot） | 011 | 100 | 1000 | 0.22 | 100 | 100 | 00 | 100 | 100 | 0.65 | 1.00 | 100 |


| FIt Permitted | 0.11 | 1.00 | 1.00 | 0.22 | 1.00 | 1.00 | 0.09 | 1.00 | 1.00 | 0.65 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| atd．Flow（perm） | 208 | 3444 | 1570 | 415 | 3614 | 1633 | 157 | 3510 | 1384 | 1140 | 1902 | 1633 |


|  | 0.93 |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| eak－hour factor，PHF | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |

RTOR Reduction（vpp）
ane Group Flow（vph）
Turn Ty
Perm NA Perm pm＋pt NA Perm pm＋pt NA Perm Perm NA Perm

| Permitted Phases | 6 | 6 | 2 | 2 | 8 | 8 | 4 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Actuated Green， $\mathrm{G}(\mathrm{s})$ | 37.0 | 37.0 | 37.0 | 60.6 | 60.6 | 60.6 | 52.7 | 52.7 | 52.7 | 42.6 | 42.6 | 42.6 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Effective Green，g（s） | 37.0 | 37.0 | 37.0 | 60.6 | 63.6 | 60.6 | 52.7 | 52.7 | 52.7 | 42.6 | 42.6 | 42.6 |


| Effective Green， $\mathrm{g}(\mathrm{s})$ | 37.0 | 37.0 | 37.0 | 60.6 | 63.6 | 60.6 | 52.7 | 52.7 | 52.7 | 42.6 | 42.6 | 42.6 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Actuated g／C Ratio | 0.29 | 0.29 | 0.29 | 0.47 | 0.50 | 0.47 | 0.41 | 0.41 | 0.41 | 0.33 | 0.33 | 0.33 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Clearance Tima（s） | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 40 | 75 | 75 | 75 | 75 | 7.5 |


|  | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Vhicle Extension（s）$\quad 3.0$
v／s Ratio Prot
$s$ Ratio Perm

| 771 | 139 | 1441 |
| :--- | :--- | :--- | :--- |


| 0.00 |
| :--- |
| 0.02 |

Uniform Delay，d1
0.00
0.02
rogression Factor $\qquad$
$\begin{array}{lr}\text { rogression Factor } & 1.00 \\ & 0.5\end{array}$
elay（s）
evel of Service
Approach Delay
Approach LOS

| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 200 Control Delay | 134.5 | HCM 2000 Level of Service | F |
| HCM 2000 Volume to Capacity ratio | 1.38 |  |  |
| Actuated Cycle Length（s） | 128.3 | Sum of lost time（s） | 23.0 |
| Intersection Capacity Utilization | $147.9 \%$ | ICU Level of Service | H |
| Analysis Period（min） | 15 |  |  |

nalysis Peripacity Utilization
c Critical Lane Group

## Timing

Existing AM Peak
3：Kennedy Road \＆Elgin Mills Road East／Elgin Mills Road Baseline

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | 7 | $\uparrow$ | F | \％ | $\uparrow$ | 「 | \％ | $\uparrow$ | 7 | \％ | $\uparrow$ | 7 |
| Trafic Volume（vph） | 11 | 176 | 86 | 63 | 589 | 11 | 116 | 229 | 7 | 14 | 621 | 85 |
| Future Volume（vph） | 11 | 176 | 86 | 63 | 589 | 11 | 116 | 229 | 7 | 14 | 621 | 85 |
| Lane Group Flow（vph） | 13 | 200 | 98 | 72 | 669 | 13 | 132 | 260 | 8 | 16 | 706 | 97 |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 | 6 |  | 6 | 2 |  |  |
| Detector Phase | 4 | 4 | 4 | 8 | 8 | 8 | 6 | 6 | 6 | 2 | 2 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Minimum Split（s） | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split（s） | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split（\％） | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ |
| Yellow Time（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All－Red Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | None | None | Max | Max | Max | Max | Max | Max |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.11 | 0.28 | 0.14 | 0.18 | 0.94 | 0.02 | 1.14 | 0.35 | 0.01 | 0.04 | 0.93 | 0.14 |
| Control Delay | 15.9 | 15.2 | 3.9 | 14.7 | 44.4 | 0.1 | 152.7 | 16.0 | 0.0 | 12.9 | 41.4 | 5.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 15.9 | 15.2 | 3.9 | 14.7 | 44.4 | 0.1 | 152.7 | 16.0 | 0.0 | 12.9 | 41.4 | 5.7 |
| Queue Length 50th（m） | 1.0 | 16.3 | 0.0 | 5.6 | 76.5 | 0.0 | ～19．6 | 22.0 | 0.0 | 1.2 | 81.3 | 1.6 |
| Queue Length 95th（m） | 4.4 | 28.7 | 7.3 | 13.1 | \＃132．8 | 0.0 | \＃47．6 | 37.1 | 0.0 | 4.3 | \＃139．8 | 9.0 |
| Internal Link Dist（m） |  | 1347.2 |  |  | 145.4 |  |  | 1762.3 |  |  | 135.7 |  |
| Turn Bay Length（m） | 100.0 |  | 50.0 | 143.0 |  | 58.0 | 142.0 |  | 48.0 | 45.0 |  | 40.0 |
| Base Capacity（vph） | 119 | 737 | 697 | 420 | 730 | 690 | 116 | 737 | 685 | 455 | 759 | 684 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Reduced v／c Ratio | 0.11 | 0.27 | 0.14 | 0.17 | 0.92 | 0.02 | 1.14 | 0.35 | 0.01 | 0.04 | 0.93 | 0.14 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 66.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 65.8 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Semi Act－Uncoord Quever shemqise maxinumaifter tye qyqles retically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |



HCM Signalized Intersection Capacity Analysis
3：Kennedy Road \＆Elgin Mills Road East／Elgin Mills Road
Existing AM Peak

|  | $y$ |  | 7 | 7 |  |  | 4 | $\uparrow$ | 7 |  |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | $\uparrow$ | 「 | \％ | $\uparrow$ | 「 | \％ | $\uparrow$ | 7 | 7 | $\uparrow$ | 「 |
| Traffic Volume（vph） | 11 | 176 | 86 | 63 | 589 | 11 | 116 | 229 | 7 | 14 | 621 | 85 |
| Future Volume（vph） | 11 | 176 | 86 | 63 | 589 | 11 | 116 | 229 | 7 | 14 | 621 | 85 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lane Util．Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd．Flow（prot） | 1825 | 1865 | 1617 | 1601 | 1847 | 1633 | 1825 | 1865 | 1633 | 1825 | 1921 | 1617 |
| Flt Permitted | 0.16 | 1.00 | 1.00 | 0.63 | 1.00 | 1.00 | 0.15 | 1.00 | 1.00 | 0.60 | 1.00 | 1.00 |
| Satd．Flow（perm） | 304 | 1865 | 1617 | 1065 | 1847 | 1633 | 296 | 1865 | 1633 | 1150 | 1921 | 1617 |
| Peak－hour factor，PHF | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 |
| Adj．Flow（vph） | 12 | 200 | 98 | 72 | 669 | 12 | 132 | 260 | 8 | 16 | 706 | 97 |
| RTOR Reduction（vph） | 0 | 0 | 60 | 0 | 0 |  | 0 | 0 | 5 | 0 | 0 | 46 |
| Lane Group Flow（vph） | 13 | 200 | 38 | 72 | 669 | 5 | 132 | 260 | 3 | 16 | 706 | 51 |
| Heavy Vehicles（\％） | 0\％ | 3\％ | 1\％ | 14\％ | 4\％ | 0\％ | 0\％ | 3\％ | 0\％ | 0\％ | 0\％ | 1\％ |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 | 6 |  | 6 | 2 |  |  |
| Actuated Green，G（s） | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 26.0 | 26.0 | 26.0 | 26.0 | 26.0 | 26.0 |
| Effective Green， $\mathrm{g}(\mathrm{s})$ | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 26.0 | 26.0 | 26.0 | 26.0 | 26.0 | 26.0 |
| Actuated g／C Ratio | 0.38 | 0.38 | 0.38 | 0.38 | 0.38 | 0.38 | 0.40 | 0.40 | 0.40 | 0.40 | 0.40 | 0.40 |
| Clearance Time（s） | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Vehicle Extension（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap（vph） | 116 | 717 | 621 | 409 | 710 | 627 | 116 | 736 | 645 | 454 | 759 | 638 |
| $\mathrm{V} / \mathrm{s}$ Ratio Prot |  | 0.11 |  |  | c0．36 |  |  | 0.14 |  |  | 0.37 |  |
| v／s Ratio Perm | 0.04 |  | 0.02 | 0.07 |  | 0.00 | c0．45 |  | 0.00 | 0.01 |  | 0.03 |
| v／c Ratio | 0.11 | 0.28 | 0.06 | 0.18 | 0.94 | 0.01 | 1.14 | 0.35 | 0.00 | 0.04 | 0.93 | 0.08 |
| Uniform Delay，d1 | 13.0 | 14.0 | 12.8 | 13.4 | 19.5 | 12.5 | 19.9 | 14.0 | 12.1 | 12.2 | 19.0 | 12.4 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay，d2 | 0.4 | 0.2 | 0.0 | 0.2 | 20.8 | 0.0 | 125.4 | 1.3 | 0.0 | 0.1 | 19.5 | 0.2 |
| Delay（s） | 13.5 | 14.2 | 12.8 | 13.6 | 40.3 | 12.5 | 145.3 | 15.3 | 12.1 | 12.4 | 38.5 | 12.7 |
| Level of Service | B | B | B | B | D | B | F | B | B | B | D |  |
| Approach Delay（s） |  | 13.7 |  |  | 37.3 |  |  | 58.2 |  |  | 35.0 |  |
| Approach LOS |  | B |  |  | D |  |  | E |  |  | C |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 36.9 |  | HCM 2000 | Level of S | ervice |  | D |  |  |  |
| HCM 2000 Control Delay HCM 2000 Volume to Capacity ratio |  |  | 1.04 |  |  |  |  |  |  |  |  |  |
|  |  |  | 65.8 |  | Sum of los | time（s） |  |  | 14.5 |  |  |  |
| $\begin{aligned} & \text { Actuated Cycle Length (s) } \\ & \text { Intersection Capacity Utilization } \end{aligned}$ |  |  | 112．9\％ |  | CU Level | Service |  |  | H |  |  |  |
|  |  |  | 15 |  |  |  |  |  |  |  |  |  |
| Analysis Period（min） <br> c Critical Lane Group |  |  |  |  |  |  |  |  |  |  |  |  |


| AMandPM＿Existing＿20220426．syn | Synchro 11 Report |
| :--- | ---: |
| R．J．Burnside \＆Associates | $05 / 20 / 2022-$ Page 6 |

4：Kennedy Road \＆Major Mackenzie Drive Baseline

|  |  |  |  |  |  |  | 4 | $\dagger$ | $p$ | \＄ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | ¢ $\uparrow$ | 7 | \％ | ¢ $\uparrow$ | F | \％ | 个个 | F | ${ }^{7}$ | 个4 | F |
| Traffic Volume（vph） | 27 | 573 | 237 | 170 | 1854 | 135 | 262 | 247 | 95 | 70 | 483 | 121 |
| Future Volume（vph） | 27 | 573 | 237 | 170 | 1854 | 135 | 262 | 247 | 95 | 70 | 483 | 121 |
| Lane Group Flow（vph） | 30 | 630 | 260 | 187 | 2037 | 148 | 288 | 271 | 104 | 77 | 531 | 133 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| Protected Phases | 1 | 6 |  | 5 | 2 |  | 3 | 8 |  |  | 4 |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 1 | 6 | 6 | 5 | 2 | 2 | ， | 8 | 8 | 4 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 7.0 | 15.0 | 15.0 | 7.0 | 15.0 | 15.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Minimum Split（s） | 11.0 | 37.5 | 37.5 | 11.0 | 37.5 | 37.5 | 11.0 | 38.5 | 38.5 | 38.5 | 38.5 | 38.5 |
| Total Split（s） | 12.0 | 63.0 | 63.0 | 12.0 | 63.0 | 63.0 | 15.0 | 55.0 | 55.0 | 40.0 | 40.0 | 40.0 |
| Total Split（\％） | 9．2\％ | 48．5\％ | 48．5\％ | 9．2\％ | 48．5\％ | 48．5\％ | 11．5\％ | 42．3\％ | 42．3\％ | 30．8\％ | 30．8\％ | 30．8\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All－Red Time（s） | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | －3．0 | 0.0 | －2．0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 4.5 | 7.5 | 2.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead |  |  | Lag | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | None | Max | Max | None | Max | Max | None | None | None | None | None | None |
| v／c Ratio | 0.18 | 0.39 | 0.30 | 0.43 | 1.08 | 0.18 | 0.91 | 0.23 | 0.17 | 0.34 | 0.74 | 0.31 |
| Control Delay | 15.1 | 23.7 | 3.5 | 16.6 | 74.6 | 5.1 | 64.4 | 30.4 | 5.9 | 45.8 | 52.1 | 8.3 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 15.1 | 23.7 | 3.5 | 16.6 | 74.6 | 5.1 | 64.4 | 30.4 | 5.9 | 45.8 | 52.1 | 8.3 |
| Queue Length 50th（m） | 2.9 | 51.8 | 0.0 | 20.0 | $\sim 304.9$ | 1.8 | 51.5 | 24.9 | 0.0 | 16.0 | 63.3 | 0.0 |
| Queue Length 95th（m） | 8.1 | 73.9 | 15.4 | 36.4 | \＃383．4 | 14.4 | \＃96．9 | 35.3 | 11.7 | 30.5 | 81.8 | 15.6 |
| Internal Link Dist（m） |  | 1479.7 |  |  | 222.6 |  |  | 139.6 |  |  | 212.5 |  |
| Turn Bay Length（ $m$ ） | 56.0 |  | 66.0 | 60.0 |  | 145.0 | 120.0 |  | 55.0 | 60.0 |  | 65.0 |
| Base Capacity（vph） | 176 | 1609 | 872 | 432 | 1891 | 839 | 315 | 1391 | 692 | 296 | 952 | 522 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.17 | 0.39 | 0.30 | 0.43 | 1.08 | 0.18 | 0.91 | 0.19 | 0.15 | 0.26 | 0.56 | 0.25 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 130 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 122.4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 150 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Semi Act－Uncoord <br>  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |


| ${ }_{01}$ | $\stackrel{\square}{\square 2}$ | 403 | $\square{ }^{1}$ |  |
| :---: | :---: | :---: | :---: | :---: |
| 12 s | 63 s | 15 s | 40 s |  |
| $\checkmark 65$ | $\rightarrow{ }_{\square}$ | 408 |  |  |
| 12 s | 63 s | 55 s |  |  |
| AMandPM＿Existing＿20220426．syn R．J．Burnside \＆Associates |  |  |  | Synchro 11 Report 05／20／2022－Page 7 |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | ¢ $\uparrow$ | F | \％ | 个个 | F | ${ }^{7}$ | $\uparrow \uparrow$ | 7 | ${ }^{7}$ | $\uparrow \uparrow$ |  |
| Trafic Volume（vph） | 27 | 573 | 237 | 170 | 1854 | 135 | 262 | 247 | 95 | 70 | 483 | 121 |
| Future Volume（vph） | 27 | 573 | 237 | 170 | 1854 | 135 | 262 | 247 | 95 | 70 | 483 | 121 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 190 |


| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Total L Lost time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 4.5 | 7.5 | 2.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |


| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 |


| Frpb，ped／bikes | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.99 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |


| $\begin{array}{lllllllllllll}\text { Frt } & 1.00 & 1.00 & 0.85 & 1.00 & 1.00 & 0.85 & 1.00 & 1.00 & 0.85 & 1.00 & 1.00\end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |


| Flt Protected | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.8 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |


|  | 1755 | 3544 | 1609 | 1789 | 3614 | 1547 | 1789 | 3579 | 1617 | 1807 | 3579 | 1595 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Satd．Flow（prot） | 0.07 | 1.00 | 1.00 | 0.34 | 1.00 | 1.00 | 0.25 | 1.00 | 1.00 | 0.58 | 1.00 | 1.00 |


| Satd．Flow（perm） | 129 | 3544 | 1609 | 633 | 3614 | 1547 | 476 | 3579 | 1617 | 1112 | 3579 | 1595 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Peak－hour factor，PHF | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Adj－Flow | 30 | 630 | 260 | 187 | 2037 | 148 | 288 | 271 | 104 | 77 | 531 | 133 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| RTOR Reduction（vph） | 0 | 030 | 140 | 0 | 0 | 68 | 0 | 0 | 71 | 0 | 0 | 1071 |


|  | 30 |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Group Flow（vph） | 30 | 630 | 120 | 187 | 2037 | 80 | 288 | 271 | 33 | 77 | 531 | 26 |
| Confl．Peds．（\＃lhr） | 2 |  | 2 | 2 |  | 2 | 1 |  |  |  |  | 10 |


|  | $4 \%$ | $3 \%$ | $0 \%$ | $2 \%$ | $1 \%$ | $3 \%$ | $2 \%$ | $2 \%$ | $1 \%$ | $1 \%$ | $2 \%$ | $1 \%$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | Turn Type | $p m+p t$ | $N A$ | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | Protected Phases


| Permitted Phases | 6 | 6 | 2 | 2 | 8 | 8 | 4 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Actuated Green，G（s） | 61.5 | 57.3 | 57.3 | 69.1 | 61.1 | 61.1 | 39.7 | 39.7 | 39.7 | 24.7 | 24.7 | 24.7 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 51.5 |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Actuated $g$／C Ratio | 0.50 | 0.46 | 0.46 | 0.56 | 0.52 | 0.49 | 0.34 | 0.32 | 0.32 | 0.70 | 0.20 | 0.20 |


|  | 7.50 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Clearance Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Vehicle Extension（s） |  |  |  |  |  |  |  |  |  |  |  |  |


|  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Grp Cap（vph） | 119 | 1637 | 743 | 427 | 1868 | 762 | 297 | 1145 | 517 | 221 | 712 | 317 |
| ／s Ratio Prot | 0.01 | 0.18 |  | c 0.03 | c 0.56 |  | c 0.10 | 0.08 |  |  | c 0.15 |  |


|  | 0.01 | 0.18 |  | c0．03 | c0．56 |  | $c 0.10$ | 0.08 |  |  | $c 0.15$ |  |
| :--- | :--- | :--- | :--- | ---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| w／s Ratio Perm | 0.12 |  | 0.07 | 0.22 |  | 0.05 | 0.22 |  | 0.02 | 0.07 |  | 0.02 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $l$ | cRatio | 0.25 | 0.38 | 0.16 | 0.44 | 1.09 | 0.11 | 0.97 | 0.24 | 0.06 | 0.35 | 0.75 |
|  | 0.027 | 0.08 |  |  |  |  |  |  |  |  |  |  |


| Uniform Delay，d1 | 27.7 | 21.8 | 19.4 | 14.3 | 30.0 | 16.8 | 36.1 | 31.0 | 29.3 | 42.7 | 46.7 | 40.4 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 1.1 | 0.7 | 0.5 | 0.7 | 50.1 | 0.3 | 43.4 | 0.1 | 0.1 | 1.0 | 4.3 | 0.1 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | C | C | B | B | F | B | E | C | C | D | D |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Apel of Service | 22.0 |  |  | 71.0 |  |  | 51.8 |  |  | 48.3 |  |


| Approach LOS | 22.0 | C E | （s）．0 | D |
| :--- | :---: | :---: | :---: | :---: |
|  |  |  |  | D |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 55.1 | HCM 2000 Level of Service | E |
| HCM 2000 Volume to Capacity ratio | 1.00 |  |  |
| Actuated Cycle Length（s） | 124.0 | Sum of lost time（s） | 21.0 |
| Intersection Capacity Utilization | $102.0 \%$ | ICU Level of Service | G |
| Analysis | 15 |  |  |

c Critical Lane Group

## AMandPM Existing 20220426．syn

AMandPM＿Existing＿2022042
Synchro 11 Repor 05／20／2022－Page 8

## Timings

1: Warden Avenue \& Elgin Mills Road East

|  | $\rangle$ |  | $\checkmark$ |  |  | 4 |  | , | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | $\hat{F}$ | \% | ¢ | ${ }^{1}$ | $\uparrow$ | F' | \% | $\uparrow$ | F |
| Traffic Volume (vph) | 19 | 293 | 137 | 681 | 79 | 149 | 11 | 6 | 830 | 80 |
| Future Volume (vph) | 19 | 293 | 137 | 681 | 79 | 149 | 11 | 6 | 830 | 80 |
| Lane Group Flow (vph) | 22 | 498 | 159 | 808 | 92 | 173 | 13 | 7 | 965 | 93 |
| Turn Type | Perm | NA | Perm | NA | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  | 8 |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 8 |  | 6 |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 10.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| Minimum Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (\%) | 52.2\% | 52.2\% | 52.2\% | 52.2\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% |
| Yellow Time (s) | 4.5 | 4.5 | 4.5 | 4.5 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 2.5 | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | -2.5 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 4.5 | 7.0 |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | Max | Max | Max | Max | Max | Max |
| v/c Ratio | 0.21 | 0.67 | 0.60 | 1.03 | 0.82 | 0.25 | 0.02 | 0.02 | 1.25 | 0.15 |
| Control Delay | 18.5 | 19.9 | 27.4 | 62.0 | 74.1 | 15.8 | 0.1 | 13.5 | 145.9 | 6.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 18.5 | 19.9 | 27.4 | 62.0 | 74.1 | 15.8 | 0.1 | 13.5 | 145.9 | 6.8 |
| Queue Length 50th (m) | 1.7 | 44.1 | 14.8 | ~104.0 | 10.1 | 14.5 | 0.0 | 0.5 | ~154.0 | 2.2 |
| Queue Length 95th ( m ) | 6.5 | 69.4 | \#33.0 | \#160.4 | \#32.7 | 25.7 | 0.0 | 2.7 | \#203.8 | 9.4 |
| Internal Link Dist (m) |  | 180.4 |  | 543.1 |  | 1650.0 |  |  | 238.4 |  |
| Turn Bay Length ( $m$ ) | 70.0 |  | 80.0 |  | 125.0 |  | 30.0 | 100.0 |  | 30.0 |
| Base Capacity (vph) | 106 | 739 | 263 | 786 | 112 | 689 | 650 | 464 | 772 | 638 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.21 | 0.67 | 0.60 | 1.03 | 0.82 | 0.25 | 0.02 | 0.02 | 1.25 | 0.15 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 67 |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 67 |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 80 |  |  |  |  |  |  |  |  |  |  |
| Control Type: Somi Act-Uncoord two cycles. <br> ~uelume exceeds capaity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |
| \#uete shown is maximum after two cycles, |  |  |  |  |  |  |  |  |  |  |



HCM Signalized Intersection Capacity Analysis
1: Warden Avenue \& Elgin Mills Road East

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | * | F |  | \% | $\uparrow$ |  | * | $\uparrow$ | F | * | $\uparrow$ |  |
| Traffic Volume (vph) | 19 | 293 | 135 | 137 | 681 | 14 | 79 | 149 | 11 | 6 | 830 |  |
| Future Volume (vph) | 19 | 293 | 135 | 137 | 681 | 14 | 79 | 149 | 11 | 6 | 830 |  |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 190 |
| Total Lost time (s) | 7.0 | 7.0 |  | 7.0 | 7.0 |  | 7.0 | 7.0 | 7.0 | 7.0 | 4.5 | 7.0 |
| Lane Utill. Factor | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.0 |
| Frt | 1.00 | 0.95 |  | 1.00 | 1.00 |  | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.8 |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 |  | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1690 | 1710 |  | 1738 | 1879 |  | 1789 | 1847 | 1633 | 1825 | 1883 | 160 |
| Flt Permitted | 0.14 | 1.00 |  | 0.35 | 1.00 |  | 0.16 | 1.00 | 1.00 | 0.65 | 1.00 | 1.0 |
| Satd. Flow (perm) | 254 | 1710 |  | 631 | 1879 |  | 301 | 1847 | 1633 | 1245 | 1883 | 160 |
| Peak-hour factor, PHF | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.8 |
| Adj. Flow (vph) | 22 | 341 | 157 | 159 | 792 | 16 | 92 | 173 | 13 | 7 | 965 |  |
| RTOR Reduction (vph) | 0 | 24 | 0 | 0 | 1 | 0 | 0 | 0 | 8 | 0 | 0 |  |
| Lane Group Flow (vph) | 22 | 474 | 0 | 159 | 807 | 0 | 92 | 173 | 5 | 7 | 965 |  |
| Heavy Vehicles (\%) | 8\% | 8\% | 5\% | 5\% | 2\% | 0\% | 2\% | 4\% | 0\% | 0\% | 2\% | 2 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | 6 |  |  | 2 |  |

Protected Phases

| Permitted Phases | 4 |  | 8 |  | 6 |  | 6 | 2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Actuated Green, G (s) | 28.0 | 28.0 | 28.0 | 28.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25. |
| Effective Green, g(s) | 28.0 | 28.0 | 28.0 | 28.0 | 25.0 | 25.0 | 25.0 | 25.0 | 27.5 | 25. |
| Actuated g/C Ratio | 0.42 | 0.42 | 0.42 | 0.42 | 0.37 | 0.37 | 0.37 | 0.37 | 0.41 | 0.3 |
| Clearance Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |  |


| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 |  | 3.0 |  | 3.0 | 3.0 | 3.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Lane Grp Cap (vph) | 106 | 7.2 | 263 | 705 | 112 | 6.9 | 609 | 464 | 772 | 597 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vs Ratio Prot |  | 0.28 |  | $c 0.43$ |  | 0.09 |  |  | 0.51 |  |

ls Ratio Perm 0.09
s Ratio Perm
Uniform D
Uniorm Delay, d1
rogression Factor
Delay (s)
evel of Service
Approach Delay (s)
Approach LOS

| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 73.5 | HCM 2000 Level of Service | E |
| HCM 2000 Volume to Capacity ratio | 1.19 | Sum of lost time (s) | 14.0 |
| Actuated Cycle Length (s) | 67.0 | (sization | $122.5 \%$ |
| Intersection Capacity Utilization of Service | H |  |  |
| Analysis Period (min) | 15 |  |  |

Critical Lane Group 2：Warden Avenue \＆Major Mackenzie Drive $\quad$ Lost Time Adjustments

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％ | ¢ $\uparrow$ | F | \％ | ¢ $\uparrow$ | 7 | \％ | ¢ $\uparrow$ | 7 | \％ | $\uparrow$ | F |
| Traffic Volume（vph） | 1 | 613 | 133 | 327 | 1844 | 19 | 62 | 146 | 61 | 30 | 994 | 88 |
| Future Volume（vph） | 1 | 613 | 133 | 327 | 1844 | 19 | 62 | 146 | 61 | 30 | 994 | 88 |
| Lane Group Flow（vph） | 1 | 659 | 143 | 352 | 1983 | 20 | 67 | 157 | 66 | 32 | 1069 | 95 |
| Turn Type | Perm | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 6 |  | 5 | 2 |  | 3 | 8 |  |  | 4 |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  |  |
| Detector Phase | 6 | 6 | 6 | 5 | 2 | 2 | 3 | 8 | 8 | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 34.0 | 34.0 | 34.0 | 7.0 | 34.0 | 34.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Minimum Split（s） | 41.5 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 41.5 | 41.5 | 41.5 |
| Total Split（s） | 43.0 | 43.0 | 43.0 | 25.0 | 68.0 | 68.0 | 12.0 | 62.0 | 62.0 | 50.0 | 50.0 | 50.0 |
| Total Split（\％） | 33．1\％ | 33．1\％ | 33．1\％ | 19．2\％ | 52．3\％ | 52．3\％ | 9．2\％ | 47．7\％ | 47．7\％ | 38．5\％ | 38．5\％ | 38．5\％ |
| Yellow Time（s） | 5.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |
| All－Red Time（s） | 2.5 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | －2．5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 7.5 | 7.5 | 7.5 | 4.0 | 5.0 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag | Lag | Lag | Lag | Lead |  |  | Lead |  |  | Lag | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes |  |  | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | Max | Max | Max | None | Max | Max | None | None | None | None | None | None |
| v／c Ratio | 0.02 | 0.66 | 0.26 | 0.85 | 1.11 | 0.02 | 0.41 | 0.11 | 0.11 | 0.08 | 1.68 | 0.15 |
| Control Delay | 36.0 | 44.5 | 6.9 | 40.4 | 89.3 | 0.1 | 28.6 | 23.5 | 5.0 | 31.2 | 344.3 | 5.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 36.0 | 44.5 | 6.9 | 40.4 | 89.3 | 0.1 | 28.6 | 23.5 | 5.0 | 31.2 | 344.3 | 5.2 |
| Queue Length 50th（m） | 0.2 | 80.1 | 0.0 | 55.6 | $\sim 310.1$ | 0.0 | 9.7 | 12.6 | 0.0 | 5.7 | $\sim 403.5$ | 0.0 |
| Queue Length 95th（m） | 1.8 | 101.2 | 15.6 | \＃96．6 | \＃351．6 | 0.0 | 18.6 | 19.8 | 7.8 | 13.4 | \＃482．2 | 10.0 |
| Internal Link Dist（m） |  | 203.6 |  |  | 501.9 |  |  | 120.0 |  |  | 316.9 |  |
| Turn Bay Length（ $m$ ） | 58.0 |  | 90.0 | 138.0 |  | 111.0 | 48.0 |  | 32.0 | 146.0 |  | 80.0 |
| Base Capacity（vph） | 60 | 1000 | 557 | 432 | 1788 | 813 | 167 | 1503 | 633 | 380 | 635 | 615 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Reduced v／c Ratio | 0.02 | 0.66 | 0.26 | 0.81 | 1.11 | 0.02 | 0.40 | 0.10 | 0.10 | 0.08 | 1.68 | 0.15 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 130 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 127.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 145 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Semi Act－Uncorrd two cycles． <br> ～Verume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Qugue shown is，maximum after two cycles．queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |


| Splits and Phases：2：Warden Avenue \＆Major M |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| $\psi_{02}$ |  | 403 | $\checkmark \square 4$ |  |
| 68 s |  | 12 s | 5 s |  |
| $\checkmark 65$ | $\rightarrow{ }_{\square}$ | 408 |  |  |
| $\underline{25 s}$ | 43 s | 62 s |  |  |
| AMandPM＿Existing＿20220426．syn Synchro 11 Report |  |  |  |  |
| R．J．Burnside \＆Associates 05／20／2022－Page 3 |  |  |  |  |

HCM Signalized Intersection Capacity Analysis
2：Warden Avenue \＆Major Mackenzie Drive

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％ | $\uparrow \uparrow$ | F | \％ | 个个 | F | \％ | 个 $\uparrow$ | 「 | ${ }^{7}$ | $\uparrow$ |  |
| Traffic Volume（vph） | 1 | 613 | 133 | 327 | 1844 | 19 | 62 | 146 | 61 | 30 | 994 |  |
| Future Volume（vph） | 1 | 613 | 133 | 327 | 1844 | 19 | 62 | 146 | 61 | 30 | 994 |  |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 7.5 | 7.5 | 7.5 | 4.0 | 5.0 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.0 |


|  | 7.5 | 7.5 | 7.5 | 4.0 | 5.0 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Total Lost time（s） | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 100 | 1.00 |


| Lane Util．Factor | 1.00 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| rtt | 0.85 |  |  |  |  |  |  |  |  |  |  |  |


|  | Flt Protected | 1.05 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | 0.95 | 1.00 | 1.00 |  |  |  |  |  |  |  |  |  |


| Satd．Flow（prot） | 1825 | 3444 | 1570 | 1772 | 3614 | 1633 | 1738 | 3510 | 1384 | 1659 | 1902 | 1633 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| FIt Permitted | 0.11 | 1.00 | 1.00 | 0.22 | 1.00 | 1.00 | 0.09 | 1.00 | 1.00 | 0.65 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Peak－hour factor．PHF 0.93

RTOR Reduction
ane Group Flow（vph）
Turn Ty
Perm NA Perm pm＋pt NA Perm pm＋pt NA Perm Perm NA Perm

| Permitted Phases | 6 | 6 | 2 | 2 | 8 | 8 | 4 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Actuated Green，G（s） | 37.0 | 37.0 | 37.0 | 60.6 | 60.6 | 60.6 | 52.7 | 52.7 | 52.7 | 42.6 | 42.6 | 42.6 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Effective Green，$g(s)$ | 37.0 | 37.0 | 37.0 | 60.6 | 63.1 | 60.6 | 52.7 | 52.7 | 52.7 | 42.6 | 42.6 | 42.6 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Actuated g／C Ratio | 0.29 | 0.29 | 0.29 | 0.47 | 0.49 | 0.47 | 0.41 | 0.41 | 0.41 | 0.33 | 0.33 | 0.33 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Clearance Tima（s） | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 40 | 75 | 75 | 75 | 75 | 7.5 |


|  | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |

v／s Ratio Prot
v／s Ratio Per $\begin{array}{lrr}771 & 139 & 1441 \\ & 00.02 & 0.04\end{array}$

.00
Uniform Delay，d1 $\qquad$

| rogression Factor | 1.00 |
| :--- | ---: |
|  | 0.5 |

Delay（s）Delay，
las
evel of Service
Approach Delay
Approach LOS

| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 200 Control Delay | 136.1 | HCM 2000 Level of Service | F |
| HCM 2000 Volume to Capacity ratio | 1.38 |  |  |
| Actuated Cycle Length（s） | 128.3 | Sum of lost time（s） | 23.0 |
| Intersection Capacity Utilization | $148.3 \%$ | ICU Level of Service | H |
| Analysis Period（min） | 15 |  |  |

sis Period（min）
Critical Lane Group

Road Existing AM with Calibration 3: Kennedy Road \& Elgin Mills Road East/Elgin Mills Road Lost Time Adjustments

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{1}$ | $\uparrow$ | 7 | ${ }^{3}$ | $\uparrow$ | 7 | \% | $\uparrow$ | F | \% | $\uparrow$ | ${ }^{7}$ |
| Traffic Volume (vph) | 11 | 176 | 86 | 63 | 589 | 11 | 116 | 229 | 7 | 14 | 621 | 85 |
| Future Volume (vph) | 11 | 176 | 86 | 63 | 589 | 11 | 116 | 229 | 7 | 14 | 621 | 85 |
| Lane Group Flow (vph) | 13 | 200 | 98 | 72 | 669 | 13 | 132 | 260 | 8 | 16 | 706 | 97 |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | - |  |  | 2 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 | 6 |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 4 | 8 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Minimum Split (s) | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split (s) | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split (\%) | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% |
| Yellow Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | -2.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 5.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | None | None | Max | Max | Max | Max | Max | Max |
| v/c Ratio | 0.11 | 0.28 | 0.14 | 0.18 | 0.94 | 0.02 | 1.11 | 0.35 | 0.01 | 0.04 | 0.93 | 0.14 |
| Control Delay | 15.9 | 15.2 | 3.9 | 14.7 | 44.4 | 0.1 | 141.0 | 16.0 | 0.0 | 12.9 | 41.4 | 5.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 15.9 | 15.2 | 3.9 | 14.7 | 44.4 | 0.1 | 141.0 | 16.0 | 0.0 | 12.9 | 41.4 | 5.7 |
| Queue Length 50th (m) | 1.0 | 16.3 | 0.0 | 5.6 | 76.5 | 0.0 | ~19.2 | 22.0 | 0.0 | 1.2 | 81.3 | 1.6 |
| Queue Length 95th (m) | 4.4 | 28.7 | 7.3 | 13.1 | \#132.8 | 0.0 | \#47.2 | 37.1 | 0.0 | 4.3 | \#139.8 | 9.0 |
| Internal Link Dist (m) |  | 1347.2 |  |  | 145.4 |  |  | 1762.3 |  |  | 135.7 |  |
| Turn Bay Length ( $m$ ) | 100.0 |  | 50.0 | 143.0 |  | 58.0 | 142.0 |  | 48.0 | 45.0 |  | 40.0 |
| Base Capacity (vph) | 119 | 737 | 697 | 420 | 730 | 690 | 119 | 737 | 685 | 455 | 759 | 684 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.11 | 0.27 | 0.14 | 0.17 | 0.92 | 0.02 | 1.11 | 0.35 | 0.01 | 0.04 | 0.93 | 0.14 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 66.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 65.8 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord two cycles. <br> Queverume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. <br> \# 95 th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |  |



HCM Signalized Intersection Capacity Analysis 3: Kennedy Road \& Elgin Mills Road East/Elgin Mills Road Lost Time Adjustment 3. Kennedy Lost Time Adjustments |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | $\mathbf{\$}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ |
| Traffic Volume (vph) | 11 | 176 | 86 | 63 | 589 | 11 | 116 | 229 | 7 | 14 | 621 | 85 |
| Future Volume (vph) | 11 | 176 | 86 | 63 | 589 | 11 | 116 | 229 | 7 | 14 | 621 | 85 | $\begin{array}{lrrrrrrrrrrrr} & 11 & 176 & 86 & 63 & 589 & 11 & 116 & 229 & 7 & 14 & 621 & 85 \\ \text { Future Volume (vph) } & 1900 & 1900 & 1900 & 1900 & 1900 & 1900 & 1900 & 1900 & 1900 & 1900 & 1900 & 1900 \\ \text { Ideal Flow (vphpl) } & 70 & 70 & 70 & 70 & 70 & 70 & 50 & 75 & 75 & 75 & 75 & 75\end{array}$ $\begin{array}{lrrrrrrrrrrrr} & 7.0 & 7.0 & 7.0 & 7.0 & 7.0 & 7.0 & 5.0 & 7.5 & 7.5 & 7.5 & 7.5 & 7.5 \\ \text { Total Lost time (s) } & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00\end{array}$ $\begin{array}{lllllllllllll}\text { Lane Util. Factor } & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 & 1.00 \\ \text { Frt } & 1.00 & 1.00 & 0.85 & 1.00 & 1.00 & 0.85 & 1.00 & 1.00 & 0.85 & 1.00 & 1.00 & 0.85\end{array}$

|  | Ift Protected | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Flt Prote | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1825 | 1865 | 1617 | 1601 | 1847 | 1633 | 1825 | 1865 | 1633 | 1825 | 1921 | 1617 |


|  | 1825 | 1865 | 1617 | 1601 | 1847 | 1633 | 1825 | 1865 | 1633 | 1825 | 1921 | 1617 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| FItt Permitted | 0.16 | 1.00 | 1.00 | 0.63 | 1.00 | 1.00 | 0.14 | 1.00 | 1.00 | 0.60 | 1.00 | 1.00 |
|  | 304 | 1005 | 1017 | 000 |  |  |  |  |  |  |  |  |


|  | 304 | 1865 | 1617 | 1065 | 1847 | 1633 | 276 | 1865 | 1633 | 1150 | 1921 | 1617 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Adj. Flow (vph) | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 |


|  | 0 | 0 | 60 | 0 | 0 | 8 | 0 | 0 | 5 | 0 | 0 | 46 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Lane Group Flow (vph) | 13 | 200 | 38 | 72 | 669 | 5 | 132 | 260 | 3 | 16 | 706 | 51 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Heavy Vehicles (\%) | $0 \%$ | $3 \%$ | $1 \%$ | $14 \%$ | $4 \%$ | $0 \%$ | $0 \%$ | $3 \%$ | $0 \%$ | $0 \%$ | $0 \%$ | $1 \%$ |

furn Type Protected Phases

| Permitted Phases | 4 | 4 | 8 | 8 | 6 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Actuated Green, $\mathrm{G}(\mathrm{s})$ | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 26.0 | 26.0 | 26.0 | 26.0 | 26.0 | 26.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Effective Green, $\mathrm{g}(\mathrm{s})$ | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 28.5 | 26.0 | 26.0 | 26.0 | 26.0 | 26.0 |


| Effective Green, $\mathrm{g}(\mathrm{s})$ | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 25.3 | 28.5 | 26.0 | 26.0 | 26.0 | 26.0 | 26.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Actuated g/C Ratio | 0.38 | 0.38 | 0.38 | 0.38 | 0.38 | 0.38 | 0.43 | 0.40 | 0.40 | 0.40 | 0.40 | 0.40 |


| Actuated g/C Ratio | 0.38 | 0.38 | 0.38 | 0.38 | 0.38 | 0.38 | 0.43 | 0.40 | 0.40 | 0.40 | 0.40 | 0.40 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Clearance Time (s) | 7.0 | 70 | 7.0 | 7.0 | 70 | 70 | 75 | 75 | 75 | 75 | 75 | 7.5 |


| Clearance Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| vehicle Extension (s) | 3.0 | 1.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Lane Grp Cap (vph) | 116 | 111 | 621 | 409 | 710 | 627 | 119 | 736 | 645 | 454 |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Ratio Perm
v/s Ratio Perm C Raio
Uiform Delay, d1
$\begin{array}{llll}0.04 & & 0.02 & 0.07 \\ 0.11 & 0.28 & 0.8\end{array}$ $\begin{array}{llll}13.0 & 14.0 & 12.8 & 13.4 \\ 1.00 & 1.06 & 18\end{array}$
cremental Delay, d2
Delay (s)
Level of Service
Approach Delay (s)
Approach LOS

| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 36.2 | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | 1.08 |  |  |
| Actuated Cycle Length (s) | 65.8 | Sum of lost time (s) | 14.5 |
| Intersection Capacity Utilization | $110.8 \%$ | ICU Level of Service | H |
| Analysis Period (min) | 15 |  |  |

c Critical Lane Group 4：Kennedy Road \＆Major Mackenzie Drive Lost Time Adjustments


| Future Volume（vph） | 27 | 573 | 237 | 170 | 1854 | 135 | 262 | 247 | 95 | 70 | 483 | 12 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Lane Group Flow（vph） | 30 | 630 | 260 | 187 | 2037 | 148 | 288 | 271 | 104 | 77 | 531 | 133 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Turn Type | pm +pt | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| Protected Phases | 1 | 6 |  | 5 | 2 |  | 3 | 8 |  |  | 4 |  |

Protected Phases

## Detector Phase

$\begin{array}{lllllllllllll}\text { Minimum Initial（s）} & 7.0 & 15.0 & 15.0 & 7.0 & 15.0 & 15.0 & 7.0 & 10.0 & 10.0 & 10.0 & 10.0 & 10.0\end{array}$

| Minimum Split（s） | 11.0 | 37.5 | 37.5 | 11.0 | 37.5 | 37.5 | 11.0 | 38.5 | 38.5 | 38.5 | 38.5 | 38.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Total Split（s） | 13.0 | 63.0 | 63.0 | 13.0 | 63.0 | 63.0 | 15.0 | 54.0 | 54.0 | 39.0 | 39.0 | 39.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Total Split（\％） | $10.0 \%$ | $48.5 \%$ | $48.5 \%$ | $10.0 \%$ | $48.5 \%$ | $48.5 \%$ | $11.5 \%$ | $41.5 \%$ | $41.5 \%$ | $30.0 \%$ | $30.0 \%$ | $30.0 \%$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Yellow Time（s） | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |


|  | 0.0 | 0.0 | 0.0 | 0.0 | -2.5 | 0.0 | -2.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Lost Time Adjust（s） | 4.0 | 7.5 | 7.5 | 4.0 | 5.0 | 7.5 | 2.0 | 7. | 7. | 7.5 | 7.5 | 7.5 |


| Total Lost Time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 5.0 | 7.5 | 2.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

Lead／Lag Lead Lag Lag Lead Lag Lag Lead $\quad$ Lag Lag Lag
Lead－Lag Optimize？Yes Yes Yes Yes Yes Yes Yes None None Yes Yes Yes
None Max Max None Max Max None None None None None None

|  | IC Ratio | 0.18 | 0.39 | 0.30 | 0.42 | 1.08 | 0.18 | 0.93 | 0.24 | 0.18 | 0.35 | 0.74 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Control Delay | 15.0 | 24.1 | 3.6 | 16.0 | 74.9 | 5.0 | 67.3 | 30.9 | 5.9 | 46.5 | 53.0 | 8.5 |


|  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Totar Delay |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue Length 50th $(\mathrm{m})$ | 25.0 | 52.1 | 3.6 | 16.0 | 74.9 | 5.0 | 67.3 | 30.9 | 5.9 | 46.5 | 53.0 | 8.5 |


| Queue Length 95th $(\mathrm{m})$ | 7.9 | 73.9 | 15.4 | 35.7 | $\# 381.5$ | 14.2 | $\# 99.6$ | 35.8 | 11.8 | 30.8 | 82.7 | 15.8 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 1479.7 |  |  | 222.6 | 145.0 | 120.0 | 139.6 |  | 55.0 | 60.0 |  | 65.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Base Capacity（vph） | 190 | 1599 | 868 | 445 | 1890 | 844 | 311 | 1353 | 675 | 284 | 916 | 507 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Spillback Cap Reductn
Storage Cap Reductn
Reduced v／c Ratio
Intersection Summary
Cycle Length： 130
Actuated Cycle Length： 123.2
Natural Cycle： 150

Qugue shown is maximum after two cycles，
Splits and Phases：4：Kennedy Road \＆Major Mackenzie Drive


|  | $\dagger$ | $\rightarrow$ | \％ | $\checkmark$ | $\leftarrow$ | 4 | 4 | $\dagger$ | 1 |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | 个个 | 7 | \％ | ¢ $\uparrow$ | 7 | ${ }^{3}$ | ¢ $\uparrow$ | 「 | \％ | ¢ $\uparrow$ | F |
| Traffic Volume（vph） | 27 | 573 | 237 | 170 | 1854 | 135 | 262 | 247 | 95 | 70 | 483 | 121 |
| Future Volume（vph） | 27 | 573 | 237 | 170 | 1854 | 135 | 262 | 247 | 95 | 70 | 483 | 121 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 5.0 | 7.5 | 2.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frpb，ped／bikes | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.99 |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd．Flow（prot） | 1755 | 3544 | 1609 | 1789 | 3614 | 1547 | 1789 | 3579 | 1617 | 1807 | 3579 | 1595 |
| Flt Permitted | 0.07 | 1.00 | 1.00 | 0.33 | 1.00 | 1.00 | 0.25 | 1.00 | 1.00 | 0.58 | 1.00 | 1.00 |
| Satd．Flow（perm） | 129 | 3544 | 1609 | 628 | 3614 | 1547 | 471 | 3579 | 1617 | 1112 | 3579 | 1595 |
| Peak－hour factor，PHF | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Adj．Flow（vph） | 30 | 630 | 260 | 187 | 2037 | 148 | 288 | 271 | 104 | 77 | 531 | 133 |
| RTOR Reduction（vph） | 0 | 0 | 141 | 0 | 0 | 67 | 0 |  | 71 |  | 0 | 107 |
| Lane Group Flow（vph） | 30 | 630 | 119 | 187 | 2037 | 81 | 288 | 271 | 33 | 77 | 531 | 26 |
| Confl．Peds．（\＃／hr） | 2 |  | 2 | 2 |  | 2 | 1 |  |  |  |  | 1 |
| Heavy Vehicles（\％） | 4\％ | 3\％ | 0\％ | 2\％ | 1\％ | 3\％ | 2\％ | 2\％ | 1\％ | 1\％ | 2\％ | 1\％ |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |


| Turn Type | $\mathrm{pm}+\mathrm{pt}$ | NA | Perm | $\mathrm{pm}+\mathrm{pt}$ | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | rotected Phases


| Permilted Phases | 6 | 6 | 6 | 2 |  | 2 | 8 | 8 | 8 | 4 |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Actuated Green，G（s） | 61.5 | 57.3 | 57.3 | 702 | 620 | 620 | 397 | 39.7 | 39 | 247 | 24.7 |


|  | actuated Green，G（s） | 61.5 | 57.3 | 57.3 | 70.2 | 62.0 | 62.0 | 39.7 | 39.7 | 39.7 | 24.7 | 24.7 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Effective Green， $\mathrm{g}(\mathrm{s})$ | 61.5 | 57.3 | 57.3 | 70.2 | 64.5 | 62.0 | 41.7 | 39.7 | 39.7 | 24.7 | 24.7 | 24.7 |


|  | 61.5 | 57.3 | 57.3 | 70.2 | 64.5 | 62.0 | 41.7 | 39.7 | 39.7 | 24.7 | 24.7 | 24.7 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Actuated $g /$ C Ratio | 0.49 | 0.46 | 0.46 | 0.56 | 0.52 | 0.50 | 0.33 | 0.32 | 0.32 | 0.20 | 0.20 | 0.20 |


|  | 0.49 | 0.46 | 0.46 | 0.50 | 0.52 | 0.50 | 0.33 | 0.32 | 0.32 | 0.20 | 0.20 |  | 0.20 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Clearance Time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 |
| Vehicle Extension（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Grp Cap（vph） | 118 | 1625 | 738 | 435 | 1866 | 767 | 294 | 1137 | 513 | 219 | 707 | 315 |
| V＇s Ratio Prot | 0.01 | 0.18 |  | $\mathrm{co.03}$ | $\mathrm{co.56}$ |  | $\mathrm{co.10}$ | 0.08 |  |  | c 0.15 |  |


| V／s Ratio Perm | 0.12 | 0.07 | 0.21 | 0.05 | 0.22 | 0.02 | 0.07 | 0.02 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| c Ratio | 0.25 | 0.39 | 0.16 | 0.43 | 1.09 | 0.10 | 0.98 | 0.24 | 0.06 | 0.35 | 0.75 | 0.08 |


| Uniform Delay，d1 | 27.9 | 22.3 | 19.8 | 14.1 | 30.2 | 16.7 | 36.7 | 31.4 | 29.7 | 43.2 | 47.2 | 40.9 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 1.1 | 0.7 | 0.5 | 0.7 | 50.6 | 0.3 | 40.3 | 0.1 | 0.1 | 1.0 | 4.5 | 0.1 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| elay（s） | 29.1 | 23.0 | 20.2 | 14.8 | 80.8 | 17.0 | 83.0 | 31.6 | 29.7 | 44.2 | 51.7 | 41.0 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| evel of Service | C | C | C | B | F | B | F | C | C | D | D | D |
| pproach Delay（s） |  | 22.4 |  |  | 71.6 |  |  | 53.6 |  |  | 49.0 |  |


| Approach Delay（s） | 22.4 | 71.6 | 53.6 | 49.0 |
| :--- | :---: | :---: | :---: | :---: |
| Approach LOS | C | E | D | D |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 55.9 | HCM 2000 Level of Service | E |
| HCM 2000 Volume to Capacity ratio | 1.00 |  |  |
| Actuated Cycle Length（s） | 124.9 | Sum of lost time（s） | 21.0 |
| Intersection Capacity Utilization | $102.4 \%$ | ICU Level of Service | G |
| Analysis Period（min） | 15 |  |  |

Timings
1: Warden Avenue \& Elgin Mills Road East


| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{*}$ | F | * | ¢ | * | $\uparrow$ | F | * | $\uparrow$ | F |
| Traffic Volume (vph) | 40 | 559 | 18 | 227 | 159 | 757 | 121 | 12 | 153 | 18 |
| Future Volume (vph) | 40 | 559 | 18 | 227 | 159 | 757 | 121 | 12 | 153 | 18 |
| Lane Group Flow (vph) | 44 | 710 | 20 | 264 | 177 | 841 | 134 | 13 | 170 | 20 |
| Turn Type | Perm | NA | Perm | NA | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  | 8 |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 8 |  | 6 |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 5.0 | 5.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| Minimum Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (\%) | 52.2\% | 52.2\% | 52.2\% | 52.2\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% |
| Yellow Time (s) | 4.5 | 4.5 | 4.5 | 4.5 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 2.5 | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |

Total Lost Time (s)
Lead-Lag Optimize?
Recall Mode None None None None Max Max Max Max Max Max

|  | 0.09 | 0.94 | 0.17 | 0.35 | 0.37 | 1.18 | 0.20 | 0.11 | 0.24 | 0.03 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 12.6 | 41.4 | 168 | 14.7 | 18.2 | 118.2 | 8.7 | 16.8 | 155 | 0.1 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Queue Length 50th $(\mathrm{m})$ | 3.2 | 78.9 | 1.5 | 21.1 | 15.7 | $\sim 130.4$ | 5.2 | 1.0 | 142 | 0.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Queue Length 50th $(\mathrm{m})$ | 3.2 | 78.9 | 1.5 | 21.1 | 15.7 | -130.4 | 5.2 | 1.0 | 14.2 | 0.0 |
| Queue Length 95th $(\mathrm{m})$ | 8.7 | $\# 144.1$ | 6.1 | 36.7 | 30.4 | $\# 191.9$ | 15.3 | 4.5 | 26.4 | 0.2 |


| Queue Length 95th ( m ) | 8.7 | $\# 144.1$ | 6.1 | 36.7 | 30.4 | $\# 191.9$ | 15.3 | 4.5 | 26.4 | 0.2 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Internal Link Dist $(m)$ |  | 180.4 |  | 543.1 |  | 1650.0 |  |  | 238.4 |  |
| Turn Bay Length $(m)$ | 70.0 |  | 80.0 |  | 125.0 |  | 30.0 | 100.0 |  | 30.0 |
| Base Capacity (vph) | 486 | 787 | 121 | 790 | 474 | 714 | 661 | 116 | 721 | 660 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Saillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.09 | 0.90 | 0.17 | 0.33 | 0.37 | 1.18 | 0.20 | 0.11 | 0.24 | 0.03 |

```
tersecle Length: 67
Actuated Cycle Length: 66
Natural Cycle: 90
```




| Splits and Phases: 1: Warden Avenue \& Elgin Mills Road East |  |  |
| :---: | :---: | :---: |
| $\dagger_{\square 0}$ | $\rightarrow_{64}$ |  |
| 32 s | 35 s |  |
| 406 | $\square_{\square 8}$ |  |
| 132 | 35 s |  |
| AMandPM_Existing_20220426.syn R.J. Burnside \& Associates |  | Synchro 11 Report 05/20/2022 - Page 1 |

HCM Signalized Intersection Capacity Analysis
1: Warden Avenue \& Elgin Mills Road East

〉 $\downarrow$

Protected Phases

| Permitted Phases | 4 | 8 | 6 |  | 6 | 2 |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Actuated Green, $\mathrm{G}(\mathrm{s})$ | 26.9 | 26.9 | 26.9 | 26.9 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Effective Green, g (s) | 26.9 | 26.9 | 26.9 | 26.9 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Act.0 |  |  |  |  |  |  |  |  |  |


| Effective Green, $g(\mathrm{~s})$ | 26.9 | 26.9 | 26.9 | 26.9 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Actuated g/C Ratio | 0.41 | 0.41 | 0.41 | 0.41 | 0.38 | 0.38 | 0.38 | 0.38 | 0.38 | 0.38 |
|  | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |


|  | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Clearance Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 30 | 3.0 | 3. |
| Vehicle Extension (s) | 3.0 |  |  |  |  |  |  |  |  |  |


| vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| ane Grp Cap (vph) | 467 | 749 | 116 | 757 | 473 | 714 | 619 | 116 | 721 |


|  | 467 | 749 | 116 | 757 | 473 | 714 | 619 | 116 | 721 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| lane Grp Cap (vph) |  | co.38 |  | 0.14 |  | $c 0.45$ |  |  | 0.09 |


| $V / s$ Ratio Perm | 0.04 | 0.07 |  | 0.14 | 0.06 | 0.04 | 0.00 |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | 0.09 | 0.94 | 0.17 | 0.35 | 0.318 | 0.15 | 0.11 | 0.24 |


| Uniform Delay, d1 | 12.0 | 18.7 | 12.4 | 13.4 | 14.8 | 20.5 | 13.4 | 13.3 | 13.9 | 12.2 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | 100 | 100 | 1.00 | 100 | 1.00 | 1.00 | 100 | 1.0 | 110 | 1.00 |


|  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| ncremental Delay, d2 | 0.1 | 19.1 | 0.7 | 0.3 | 2.3 | 94.2 | 0.5 | 1.9 | 0.8 | 0.0 |


|  | B | D | B | B | B | F | B | B | B |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Level of Service |  |  | 13.7 |  | 88.0 |  |  | 14.6 |  |
| Approach Delay (s) |  | 36.3 | D |  | B |  | F |  |  |
| Approach LOS |  |  |  |  |  |  |  |  |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 200 Control Delay | 56.6 | HCM 2000 Level of Service | E |
| HCM 2000 Volume to Capacity ratio | 1.05 | Sum of lost time (s) | 14.0 |
| Actuated Cycle Length (s) | 65.9 | ICU Level of Service | G |
| Intersection Capacity Utilization | $104.1 \%$ |  |  |

c Critical Lane Group

2: Warden Avenue \& Major Mackenzie Drive

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | ¢ $\uparrow$ | F | \% | ¢ $\uparrow$ | F | * | ¢ $\uparrow$ | F | \% | $\uparrow$ | 「 |
| Traffic Volume (vph) | 33 | 1783 | 120 | 104 | 670 | 29 | 200 | 967 | 314 | 27 | 179 | 16 |
| Future Volume (vph) | 33 | 1783 | 120 | 104 | 670 | 29 | 200 | 967 | 314 | 27 | 179 | 16 |
| Lane Group Flow (vph) | 36 | 1938 | 130 | 113 | 728 | 32 | 217 | 1051 | 341 | 29 | 195 | 17 |
| Turn Type | Perm | NA | Perm | pm+pt | NA | Perm | pm+pt | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 6 |  | 5 | 2 |  | 3 | 8 |  |  | 4 |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 6 | 6 | 6 | 5 | 2 | 2 | 3 | 8 | 8 | 4 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 34.0 | 34.0 | 34.0 | 7.0 | 34.0 | 34.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Minimum Split (s) | 41.5 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 41.5 | 41.5 | 41.5 |
| Total Split (s) | 63.0 | 63.0 | 63.0 | 12.0 | 75.0 | 75.0 | 12.0 | 55.0 | 55.0 | 43.0 | 43.0 | 43.0 |
| Total Split (\%) | 48.5\% | 48.5\% | 48.5\% | 9.2\% | 57.7\% | 57.7\% | 9.2\% | 42.3\% | 42.3\% | 33.1\% | 33.1\% | 33.1\% |
| Yellow Time (s) | 5.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |
| All-Red Time (s) | 2.5 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead/Lag | Lag | Lag | Lag | Lead |  |  | Lead |  |  | Lag | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes |  |  | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | Max | Max | Max | None | Max | Max | None | None | None | None | None | None |
| v/c Ratio | 0.11 | 1.20 | 0.17 | 0.65 | 0.37 | 0.04 | 0.53 | 0.83 | 0.53 | 0.48 | 0.41 | 0.03 |
| Control Delay | 23.6 | 129.6 | 6.8 | 37.6 | 18.2 | 0.1 | 33.8 | 44.6 | 21.8 | 68.0 | 41.9 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 23.6 | 129.6 | 6.8 | 37.6 | 18.2 | 0.1 | 33.8 | 44.6 | 21.8 | 68.0 | 41.9 | 0.1 |
| Queue Length 50th (m) | 5.5 | $\sim 324.6$ | 3.7 | 13.5 | 57.0 | 0.0 | 38.0 | 125.4 | 39.1 | 6.2 | 40.5 | 0.0 |
| Queue Length 95th (m) | 12.8 | \#366.3 | 15.6 | \#36.3 | 71.3 | 0.3 | 57.8 | 151.7 | 67.4 | \#18.1 | 62.3 | 0.0 |
| Internal Link Dist (m) |  | 203.6 |  |  | 501.9 |  |  | 120.0 |  |  | 316.9 |  |
| Turn Bay Length ( $m$ ) | 58.0 |  | 90.0 | 138.0 |  | 111.0 | 48.0 |  | 32.0 | 146.0 |  | 80.0 |
| Base Capacity (vph) | 318 | 1613 | 773 | 176 | 1954 | 882 | 408 | 1375 | 696 | 68 | 540 | 535 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.11 | 1.20 | 0.17 | 0.64 | 0.37 | 0.04 | 0.53 | 0.76 | 0.49 | 0.43 | 0.36 | 0.03 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 130 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 126.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 145 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord two cycles. <br> Quevarime exceeds capacify, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |  |  |
| \#ugut shown is maximum after two crcles, |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: $\quad$ 2: Warden Avenue \& Major Mackenzie Drive


|  | $\dagger$ |  |  | 7 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | ¢ $\uparrow$ | \% | 7 | ¢ $\uparrow$ | F' | \% | 个 $\uparrow$ | 7 | \% | $\uparrow$ |  |
| Traffic Volume (vph) | 33 | 1783 | 120 | 104 | 670 | 29 | 200 | 967 | 314 | 27 | 179 |  |
| Future Volume (vph) | 33 | 1783 | 120 | 104 | 670 | 29 | 200 | 967 | 314 | 27 | 179 | 16 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lane Util. Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1825 | 3650 | 1617 | 1789 | 3650 | 1585 | 1825 | 3650 | 1617 | 1825 | 1921 | 1633 |
| Flt Permitted | 0.38 | 1.00 | 1.00 | 0.07 | 1.00 | 1.00 | 0.49 | 1.00 | 1.00 | 0.13 | 1.00 | 1.00 |
| Satd. Flow (perm) | 720 | 3650 | 1617 | 126 | 3650 | 1585 | 946 | 3650 | 1617 | 243 | 1921 | 1633 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 36 | 1938 | 130 | 113 | 728 | 32 | 217 | 1051 | 341 | 29 | 195 | 17 |
| RTOR Reduction (vph) | , | , | 59 | 0 | 0 | 15 | 0 | , | 91 | - | 0 | 1 |
| Lane Group Flow (vph) | 36 | 1938 | 71 | 113 | 728 | 17 | 217 | 1051 | 250 | 29 | 195 |  |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 2\% | 0\% | 3\% | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% |
| Turn Type | Perm | NA | Perm | pm+pt | NA | Perm | pm+pt | NA | Perm | Perm | NA | Per |
| Protected Phases |  | 6 |  | 5 | 2 |  | 3 | 8 |  |  | 4 |  |
| Permitted Phases | , |  | . | 2 |  | 2 | 8 |  | 8 | 4 |  |  |
| Actuated Green, G (s) | 55.8 | 55.8 | 55.8 | 67.6 | 67.6 | 67.6 | 43.6 | 43.6 | 43.6 | 31.6 | 31.6 | 31. |
| Effective Green, g (s) | 55.8 | 55.8 | 55.8 | 67.6 | 67.6 | 67.6 | 43.6 | 43.6 | 43.6 | 31.6 | 31.6 | 31. |
| Actuated g/C Ratio | 0.44 | 0.44 | 0.44 | 0.54 | 0.54 | 0.54 | 0.35 | 0.35 | 0.35 | 0.25 | 0.25 | 0.2 |
| Clearance Time (s) | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 318 | 1613 | 714 | 170 | 1955 | 849 | 382 | 1261 | 558 | 60 | 481 | 408 |
| $\mathrm{V} / \mathrm{s}$ Ratio Prot |  | c0.53 |  | c0.04 | 0.20 |  | 0.04 | c0.29 |  |  | 0.10 |  |
| v/s Ratio Perm | 0.05 |  | 0.04 | 0.31 |  | 0.01 | 0.16 |  | 0.15 | 0.12 |  | 0.0 |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.11 | 1.20 | 0.10 | 0.66 | 0.37 | 0.02 | 0.57 | 0.83 | 0.45 | 0.48 | 0.41 | 0.0 |
| Uniform Delay, d1 | 20.7 | 35.2 | 20.5 | 27.6 | 17.0 | 13.8 | 33.0 | 38.0 | 32.0 | 40.3 | 39.5 | 35 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.0 |
| Incremental Delay, d2 | 0.7 | 96.9 | 0.3 | 9.4 | 0.5 | 0.0 | 1.9 | 4.9 | 0.6 | 6.0 | 0.6 | 0.0 |
| Delay (s) | 21.4 | 132.1 | 20.8 | 37.1 | 17.5 | 13.8 | 34.9 | 42.8 | 32.6 | 46.4 | 40.0 | 35. |
| Level of Service | c | F | c | D | B | B | C | D | c | D | D |  |
| Approach Delay (s) |  | 123.3 |  |  | 19.9 |  |  | 39.6 |  |  | 40.5 |  |
| Approach LOS |  | F |  |  | B |  |  | D |  |  | D |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 2000 Control Delay |  |  | 72.6 |  | HCM 2000 | Level of S | Service |  | E |  |  |  |
|  |  |  | 1.05 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 126.2 |  | Sum of los | time (s) |  |  | 23.0 |  |  |  |
| Intersection Capacity Utilization |  |  | 116.7\% |  | CU Level | fervice |  |  | H |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

Timings
3: Kennedy Road \& Elgin Mills Road East/Elgin Mills Road

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\uparrow$ | F | \% | $\uparrow$ | 「 | \% | $\uparrow$ | 7 | 7 | $\uparrow$ | 7 |
| Traffic Volume (vph) | 47 | 543 | 102 | 16 | 184 | 6 | 90 | 386 | 44 | 12 | 243 | 13 |
| Future Volume (vph) | 47 | 543 | 102 | 16 | 184 | 6 | 90 | 386 | 44 | 12 | 243 | 13 |
| Lane Group Flow (vph) | 48 | 560 | 105 | 16 | 190 | 6 | 93 | 398 | 45 | 12 | 251 | 13 |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 | 6 |  | 6 | 2 |  |  |
| Detector Phase | 4 | 4 | 4 | 8 | 8 | 8 | 6 | 6 | 6 | 2 | 2 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Minimum Split (s) | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split (s) | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split (\%) | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% |
| Yellow Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | None | None | Max | Max | Max | Max | Max | Max |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.11 | 0.83 | 0.17 | 0.10 | 0.28 | 0.01 | 0.19 | 0.51 | 0.07 | 0.03 | 0.32 | 0.02 |
| Control Delay | 13.8 | 30.2 | 4.0 | 15.0 | 15.4 | 0.0 | 14.6 | 17.6 | 2.7 | 13.0 | 15.0 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 13.8 | 30.2 | 4.0 | 15.0 | 15.4 | 0.0 | 14.6 | 17.6 | 2.7 | 13.0 | 15.0 | 0.1 |
| Queue Length 50th (m) | 3.7 | 57.9 | 0.0 | 1.2 | 15.4 | 0.0 | 7.4 | 36.5 | 0.0 | 0.9 | 21.0 | 0.0 |
| Queue Length 95th (m) | 9.6 | \#96. 2 | 7.9 | 4.9 | 28.2 | 0.0 | 16.4 | 60.0 | 3.5 | 3.8 | 36.8 | 0.0 |
| Internal Link Dist (m) |  | 1347.2 |  |  | 145.4 |  |  | 1762.3 |  |  | 135.7 |  |
| Turn Bay Length ( $m$ ) | 100.0 |  | 50.0 | 143.0 |  | 58.0 | 142.0 |  | 48.0 | 45.0 |  | 40.0 |
| Base Capacity (vph) | 506 | 785 | 715 | 178 | 785 | 717 | 477 | 785 | 668 | 369 | 785 | 712 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Reduced v/c Ratio | 0.09 | 0.71 | 0.15 | 0.09 | 0.24 | 0.01 | 0.19 | 0.51 | 0.07 | 0.03 | 0.32 | 0.02 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 66.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 63.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 70 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control TyRe: Semi Actuncond to crcles <br> \# Gbth percentile volume exceeds cappacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |  |



| AMandPM_Existing_20220426.syn | Synchro 11 Report |
| :--- | ---: |
| R.J. Burnside \& Associates | $05 / 20 / 2022$ - Page 5 |

Timings
4：Kennedy Road \＆Major Mackenzie Drive


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％ | ¢ $\uparrow$ | F | \％ | 个 $\uparrow$ | F | \％ | 个个 | 「 | \％ | 个个 | 「 |
| Traffic Volume（vph） | 37 | 1858 | 233 | 81 | 773 | 32 | 218 | 323 | 160 | 81 | 244 | 26 |
| Future Volume（vph） | 37 | 1858 | 233 | 81 | 773 | 32 | 218 | 323 | 160 | 81 | 244 | 26 |
| Lane Group Flow（vph） | 38 | 1896 | 238 | 83 | 789 | 33 | 222 | 330 | 163 | 83 | 249 | 27 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| Protected Phases | 1 | 6 |  | 5 | 2 |  | 3 | 8 |  |  | ， |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 1 | 6 | 6 | 5 | 2 | 2 | 3 | 8 | 8 | 4 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 7.0 | 15.0 | 15.0 | 7.0 | 15.0 | 15.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Minimum Split（s） | 11.0 | 37.5 | 37.5 | 11.0 | 37.5 | 37.5 | 11.0 | 38.5 | 38.5 | 38.5 | 38.5 | 38.5 |
| Total Split（s） | 12.0 | 66.0 | 66.0 | 12.0 | 66.0 | 66.0 | 12.0 | 52.0 | 52.0 | 40.0 | 40.0 | 40.0 |
| Total Split（\％） | 9．2\％ | 50．8\％ | 50．8\％ | 9．2\％ | 50．8\％ | 50．8\％ | 9．2\％ | 40．0\％ | 40．0\％ | 30．8\％ | 30．8\％ | 30．8\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All－Red Time（s） | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead |  |  | Lag | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | None | Max | Max | None | Max | Max | None | None | None | None | None | None |
| $\mathrm{v} / \mathrm{R}$ Ratio | 0.08 | 0.97 | 0.26 | 0.44 | 0.39 | 0.04 | 0.70 | 0.37 | 0.33 | 0.58 | 0.52 | 0.09 |
| Control Delay | 8.2 | 40.3 | 8.6 | 19.4 | 15.6 | 0.1 | 47.3 | 36.0 | 10.7 | 62.4 | 48.4 | 0.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 8.2 | 40.3 | 8.6 | 19.4 | 15.6 | 0.1 | 47.3 | 36.0 | 10.7 | 62.4 | 48.4 | 0.6 |
| Queue Length 50th（m） | 2.6 | 203.0 | 12.9 | 5.8 | 51.4 | 0.0 | 40.3 | 31.7 | 4.9 | 17.3 | 27.0 | 0.0 |
| Queue Length 95th（m） | 7.5 | \＃300．0 | 31.0 | 19.0 | 76.9 | 0.0 | 62.3 | 44.7 | 21.0 | 33.5 | 39.6 | 0.0 |
| Internal Link Dist（m） |  | 1479.7 |  |  | 222.6 |  |  | 139.6 |  |  | 212.5 |  |
| Turn Bay Length（ $m$ ） | 56.0 |  | 66.0 | 60.0 |  | 145.0 | 120.0 |  | 55.0 | 60.0 |  | 65.0 |
| Base Capacity（vph） | 473 | 1959 | 918 | 196 | 2025 | 941 | 315 | 1490 | 737 | 316 | 1077 | 554 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Reduced v／c Ratio | 0.08 | 0.97 | 0.26 | 0.42 | 0.39 | 0.04 | 0.70 | 0.22 | 0.22 | 0.26 | 0.23 | 0.05 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 130 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 109.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control TyRe Semi Act－Uncord two cycles． <br> \＃G5th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |



| AMandPM＿Existing＿20220426．syn | Synchro 11 Report <br> $05 / 20 / 2022$ |
| :--- | ---: |
| R．Page 7 |  |

R．J．Burnside \＆Associates
Synchro 11 Repo
55／20／2022－Page 7
 4：Kennedy Road \＆Major Mackenzie Drive

| P | $\downarrow$ | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: |
| NBR | SBL | SBT | SBR |
| $\overline{7}$ | \％ | 个 $\uparrow$ | F |
| 160 | 81 | 244 | 26 |
| 160 | 81 | 244 | 26 |
| 900 | 1900 | 1900 | 1900 |
| 7.5 | 7.5 | 7.5 | 7.5 |
| 1.00 | 1.00 | 0.95 | 1.00 |
| 0.99 | 1.00 | 1.00 | 0.99 |
| 1.00 | 1.00 | 1.00 | 1.00 |
| 0.85 | 1.00 | 1.00 | 0.85 |
| 1.00 | 0.95 | 1.00 | 1.00 |
| 612 | 1824 | 3614 | 1612 |
| 1.00 | 0.55 | 1.00 | 1.00 |
| 612 | 1061 | 3614 | 1612 |
| 0.98 | 0.98 | 0.98 | 0.98 |
| 163 | 83 | 249 | 27 |
| 102 | 0 | 0 | 23 |
| 61 | 83 | 249 | 4 |
| 1 | 1 |  | 1 |
| 0\％ | 0\％ | 1\％ | 0\％ |
| erm | Perm | NA | Perm |
|  |  | 4 |  |
| 8 | 4 |  | 4 |
| 26.7 | 14.7 | 14.7 | 14.7 |
| 26.7 | 14.7 | 14.7 | 14.7 |
| 0.24 | 0.13 | 0.13 | 0.13 |
| 7.5 | 7.5 | 7.5 | 7.5 |
| 3.0 | 3.0 | 3.0 | 3.0 |
| 387 | 140 | 477 | 213 |
|  |  | 0.07 |  |
| 0.04 | 0.08 |  | 0.00 |
| 0.16 | 0.59 | 0.52 | 0.02 |
| 33.4 | 45.4 | 45.0 | 42.0 |
| 1.00 | 1.00 | 1.00 | 1.00 |
| 0.2 | 6.6 | 1.0 | 0.0 |
| 33.6 | 52.0 | 46.0 | 42.0 |
| c | D | D | D |
|  |  | 47.1 |  |
|  |  | D |  |
|  |  |  |  |
|  |  |  |  |
|  |  |  |  |
| 23.0 |  |  |  |

Analysis Period（min）
c Critical Lane Group

1: Warden Avenue \& Elgin Mills Road East
Existing PM with Calibration

|  | $\rangle$ |  | $\dagger$ |  | 4 | $\uparrow$ |  |  |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\dagger$ | \% | A | ${ }^{*}$ | $\uparrow$ | 7 | * | $\uparrow$ | F |
| Traffic Volume (vph) | 40 | 559 | 18 | 227 | 159 | 757 | 121 | 12 | 153 | 18 |
| Future Volume (vph) | 40 | 559 | 18 | 227 | 159 | 757 | 121 | 12 | 153 | 18 |
| Lane Group Flow (vph) | 44 | 710 | 20 | 264 | 177 | 841 | 134 | 13 | 170 | 20 |
| Turn Type | Perm | NA | Perm | NA | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  | 8 |  | 6 |  |  | 2 |  |
| Permitted Phases | , |  | 8 |  |  |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 10.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| Minimum Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (\%) | 52.2\% | 52.2\% | 52.2\% | 52.2\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% |
| Yellow Time (s) | 4.5 | 4.5 | 4.5 | 4.5 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 2.5 | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | -2.5 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 4.5 | 7.0 | 7.0 | 7.0 | 7.0 |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | Max | Max | Max | Max | Max | Max |
| v/c Ratio | 0.09 | 0.94 | 0.17 | 0.35 | 0.37 | 1.07 | 0.20 | 0.11 | 0.24 | 0.03 |
| Control Delay | 12.6 | 41.4 | 16.8 | 14.7 | 18.2 | 75.9 | 8.7 | 16.8 | 15.5 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 12.6 | 41.4 | 16.8 | 14.7 | 18.2 | 75.9 | 8.7 | 16.8 | 15.5 | 0.1 |
| Queue Length 50th (m) | 3.2 | 78.9 | 1.5 | 21.1 | 15.7 | ~121.0 | 5.2 | 1.0 | 14.2 | 0.0 |
| Queue Length 95th (m) | 8.7 | \#144.1 | 6.1 | 36.7 | 30.4 | \#182.5 | 15.3 | 4.5 | 26.4 | 0.2 |
| Internal Link Dist (m) |  | 180.4 |  | 543.1 |  | 1650.0 |  |  | 238.4 |  |
| Turn Bay Length ( m ) | 70.0 |  | 80.0 |  | 125.0 |  | 30.0 | 100.0 |  | 30.0 |
| Base Capacity (vph) | 486 | 787 | 121 | 790 | 474 | 785 | 661 | 116 | 721 | 660 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.09 | 0.90 | 0.17 | 0.33 | 0.37 | 1.07 | 0.20 | 0.11 | 0.24 | 0.03 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 67 |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 66 |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 90 |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord two cycles. <br> Quegertume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |
| Queut shown is maximum after two cycles <br> \# 55 h percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |



HCM Signalized Intersection Capacity Analysis
1: Warden Avenue \& Elgin Mills Road East

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\hat{F}$ |  | \% | $\hat{F}$ |  | \% | $\uparrow$ | F' | 7 | $\uparrow$ | T |
| Traffic Volume (vph) | 40 | 559 | 80 | 18 | 227 | 11 | 159 | 757 | 121 | 12 | 153 |  |
| Future Volume (vph) | 40 | 559 | 80 | 18 | 227 | 11 | 159 | 757 | 121 | 12 | 153 |  |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 7.0 | 7.0 |  | 7.0 | 7.0 |  | 7.0 | 4.5 | 7.0 | 7.0 | 7.0 | 7.0 |
| Lane Util. Factor | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 0.98 |  | 1.00 | 0.99 |  | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 |  | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1825 | 1837 |  | 1825 | 1855 |  | 1825 | 1883 | 1633 | 1825 | 1902 | 1633 |
| Flt Permitted | 0.60 | 1.00 |  | 0.15 | 1.00 |  | 0.65 | 1.00 | 1.00 | 0.16 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1146 | 1837 |  | 286 | 1855 |  | 1248 | 1883 | 1633 | 307 | 1902 | 1633 |
| Peak-hour factor, PHF | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Adj. Flow (vph) | 44 | 621 | 89 | 20 | 252 | 12 | 177 | 841 | 134 | 13 | 170 | 20 |
| RTOR Reduction (vph) | 0 | 8 | 0 | 0 | 2 | , | 0 | 0 | 42 | 0 | - |  |
| Lane Group Flow (vph) | 44 | 702 | 0 | 20 | 262 | 0 | 177 | 841 | 92 | 13 | 170 |  |
| Heavy Vehicles (\%) | 0\% | 3\% | 0\% | 0\% | 3\% | 0\% | 0\% | 2\% | 0\% | 0\% | 1\% | 0\% |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | 6 |  |  | 2 |  |


| Protected Phases | 4 | 4 | 8 | 8 | 6 | 6 |  | 2 |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Permitted Phases | 4 |  | 8 |  | 6 |  |  |  |  |


| Permitted Phases |  | 4 | 8 |  | 6 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Actuated Green, $\mathrm{G}(\mathrm{s})$ | 26.9 | 26.9 | 26.9 | 26.9 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Effective Green $\mathrm{g}(\mathrm{s})$ | 26.9 | 26.9 | 26.9 | 26.9 | 250 | 27.5 | 25.0 | 25.0 | 25.0 | 25.0 |


| Effective Green, $\mathrm{g}(\mathrm{s})$ | 26.9 | 26.9 | 26.9 | 26.9 | 25.0 | 27.5 | 25.0 | 25.0 | 25.0 | 25.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Actuated $\mathrm{g} / \mathrm{C}$ Ratio | 0.41 | 0.41 | 0.41 | 0.41 | 0.38 | 0.42 | 0.38 | 0.38 | 0.38 | 0.38 |


| Clearance Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| ane Grp Cap (vph) | 467 | 749 | 116 | 757 | 473 | 785 | 619 | 116 | 721 | 619 |


|  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| lane Grp Cap (vph) | 467 | 749 | 116 | 757 | 473 | 785 | 619 | 116 | 721 |
| v/s Ratio Prot |  | 0.38 |  | 0.14 |  | $c 0.45$ |  |  | 0.09 |


| W/s Ratio Perm | 0.04 | 0.07 |  | 0.14 | 0.06 | 0.04 | 0.00 |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | 0.09 | 0.94 | 0.17 | 0.35 | 0.37 | 1.07 | 0.15 | 0.11 |


| Uniform Delay, d1 | 12.0 | 18.7 | 12.4 | 13.4 | 14.8 | 19.2 | 13.4 | 13.3 | 13.9 | 12. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Joression Factor | 0 | 0 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |


| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Incremental Delay, d2 | 0.1 | 19.1 | 0.7 | 0.3 | 2.3 | 53.0 | 0.5 | 1.9 | 0.8 |
|  | 1.0 | 0.0 |  |  |  |  |  |  |  |

Delay (s) Deay, 12
evel of Service
Approach Delay (s)
Approach LOS

| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 200 Control Delay | 41.7 | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | 1.05 |  | 14.0 |
| Actuated Cycle Length (s) | 65.9 | Sum of lost time (s) | G |
| Intersection Capacity Utilization | $102.0 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |

Critical Lane Grou

Timings
Existing PM with Calibration Lost Time Adjustments 2：Warden Avenue \＆Major Mackenzie Drive


| Lane Group Flow（vph） | 36 | 1938 | 130 | 113 | 728 | 32 | 217 | 1051 | 341 | 29 | 195 | 1 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Turn Type | Perm | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Proteted Phases | 6 | 6 | 6 | 5 | 2 | 2 | 3 | 8 | 8 | 4 |  | 4 |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  |  | eetector Phase

Switch Phase
$\begin{array}{lllllllllllll}\text { Minimum Initial（s）} & 34.0 & 34.0 & 34.0 & 7.0 & 34.0 & 34.0 & 7.0 & 10.0 & 10.0 & 10.0 & 10.0 & 10.0\end{array}$

| Minimum Split（s） | 41.5 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 41.5 | 41.5 | 41.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Total Split（s） | 63.0 | 63.0 | 63.0 | 12.0 | 75.0 | 75.0 | 12.0 | 55.0 | 550 | 43.0 | 43.0 | 43.0 |


| Total Split（\％） | $48.5 \%$ | $48.5 \%$ | $48.5 \%$ | $9.2 \%$ | $57.7 \%$ | $57.7 \%$ | $9.2 \%$ | $42.3 \%$ | $42.3 \%$ | $33.1 \%$ | $33.1 \%$ | $33.1 \%$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Yellow Time（s） | 5.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| All－Red Time（s） | 2.5 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Ost Time Adjust（s） | 0.0 | -2.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Total Lost Time（s） | 7.5 | 5.0 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lead／Lag | Lag | Lag | Lag | Lead |  |  | Lead |  |  | Lag | Lag | Lag |


| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes |  |  | Yes |  |  | Yes | Yes | Yes |
| :--- | ---: | ---: | ---: | ---: | :--- | :--- | :--- | :--- | :--- | ---: | ---: | ---: |
| Recall Mode | Max | Max | Max | None | Max | Max | None | None | None | None | None | None |


| IC Ratio | 0.11 | 1.15 | 0.17 | 0.65 | 0.37 | 0.04 | 0.53 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 23.6 | 107.4 | 6.8 | 37.6 | 10.2 | 0.1 | 3.8 | 44.6 | 21.7 | 68.0 | 41.9 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 0.1 |  |  |  |  |  |  |  |  |  |  |  |


| total Delay | 23.6 | 107.4 | 6.8 | 37.6 | 18.2 | 0.1 | 33.8 | 44.6 | 21.7 | 68.0 | 41.9 | 0 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Queue Length 50 th $(m)$ | 5.5 | $\sim 314.7$ | 3.7 | 13.5 | 57.0 | 0.0 | 38.0 | 125.4 | 38.9 | 6.2 | 40.5 | 0.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Queue Length 95th $(\mathrm{m})$ | 12.8 | $\# 356.5$ | 15.6 | $\# 36.3$ | 71.3 | 0.3 | 57.8 | 151.7 | 67.3 | $\# 18.1$ | 62.3 | 0.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  |  | 203.6 |  | 12.0 | 501.9 | 111.0 | 48.0 | 120.0 |  | 32.0 | 146.0 | 316.9 | 80.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Base Capacity（vph） | 318 | 1685 | 773 | 176 | 1954 | 882 | 408 | 1375 | 696 | 68 | 540 | 535 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Storage Cap Reduc

## ntersection Summan

## ycle Length： 130

Actuated Cycle Len




| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{4}$ | 个4 | 「 | \％ | ¢个 | 7 | ${ }^{1}$ | 个4 | F＇ | \％ | $\uparrow$ |  |
| Traffic Volume（vph） | 33 | 1783 | 120 | 104 | 670 | 29 | 200 | 967 | 314 | 27 | 179 |  |
| Future Volume（vph） | 33 | 1783 | 120 | 104 | 670 | 29 | 200 | 967 | 314 | 27 | 179 |  |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time（s） | 7.5 | 5.0 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd．Flow（prot） | 1825 | 3650 | 1617 | 1789 | 3650 | 1585 | 1825 | 3650 | 1617 | 1825 | 1921 | 1633 |
| Flt Permitted | 0.38 | 1.00 | 1.00 | 0.07 | 1.00 | 1.00 | 0.49 | 1.00 | 1.00 | 0.13 | 1.00 | 1.00 |
| Satd．Flow（perm） | 720 | 3650 | 1617 | 126 | 3650 | 1585 | 946 | 3650 | 1617 | 243 | 1921 | 1633 |
| Peak－hour factor，PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj．Flow（vph） | 36 | 1938 | 130 | 113 | 728 | 32 | 217 | 1051 | 341 | 29 | 195 |  |
| RTOR Reduction（vph） | ， | 0 | 59 | 0 | 0 | 15 | ， | 0 | 92 | 0 | 0 |  |
| Lane Group Flow（vph） | 36 | 1938 | 71 | 113 | 728 | 17 | 217 | 1051 | 249 | 29 | 195 |  |
| Heavy Vehicles（\％） | 0\％ | 0\％ | 1\％ | 2\％ | 0\％ | 3\％ | 0\％ | 0\％ | 1\％ | 0\％ | 0\％ | 0\％ |
| Turn Type | Perm | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Per |
| Protected Phases |  | 6 |  | 5 | 2 |  |  | 8 |  |  | 4 |  |

Potected Phases

| Permitted Phases | 6 | 6 | 2 | 2 | 8 | 8 | 4 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

$\begin{array}{lllllllllllll}\text { Actuated Green，} \mathrm{G}(\mathrm{s}) & 55.8 & 55.8 & 55.8 & 67.6 & 67.6 & 67.6 & 43.6 & 43.6 & 43.6 & 31.6 & 31.6 & 31.6 \\ \text { Effective Green，} \mathrm{g}(\mathrm{s}) & 55.8 & 58.3 & 55.8 & 67.6 & 67.6 & 67.6 & 43.6 & 43.6 & 43.6 & 31.6 & 31.6 & 31.6\end{array}$

| Effective Green，$g(s)$ | 55.8 | 58.3 | 55.8 | 67.6 | 67.6 | 67.6 | 43.6 | 43.6 | 43.6 | 31.6 | 31.6 | 31.6 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Actuaved |  | 0.44 | 0.46 | 0.44 | 0.54 | 0.54 | 0.54 | 0.35 | 035 | 0.35 | 0.25 | 0.25 |
| 0.250 .6 |  |  |  |  |  |  |  |  |  |  |  |  |


| Actuated g／C Ratio | 0.44 | 0.46 | 0.44 | 0.54 | 0.54 | 0.54 | 0.35 | 0.35 | 0.35 | 0.25 | 0.25 | 0.25 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Cluarance Time（s） | 7.5 | 7.5 | 75 | 40 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |


| Clearance Time（s） | 7.5 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 3.0 | 3.0 |  | 0.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 318 | 0.53 | 714 | 0.04 | 0.20 | 849 | 382 | 1201 | 558 | 60 | 4010 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Ratio Perm
s Ratio Per
Ratio
0.05 co．53
$\qquad$

|  | 20.7 |
| :--- | :--- |
| Progression Factor | 100 | |  | 1.15 | 0.10 | 0.66 | 0.37 | 0.01 | 0.16 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Uniform Delay，d1 | 20.7 | 34.0 | 20.5 | 27.6 | 17.0 | 13.8 | 33.0 | 38.0 | 32.0 | 40.3 | 39.5 | 35 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| sion Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |

ental Delay，d2
（s）
Level of Service
Approach Delay

| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 63.1 | HCM 2000 Level of Service | E |
| HCC 2000 VVlume to Capacity ratio | 1.03 | Sum of lost time（s） | 20.5 |
| Actuated Cycle Length（s） | 126.2 | H |  |
| Intersection Capacity Utilization | $116.7 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |

Existing PM with Calibration 3: Kennedy Road \& Elgin Mills Road East/Elgin Mills Road Lost Time Adjustments

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{*}$ | $\uparrow$ | F' | * | $\uparrow$ | F' | ${ }^{7}$ | $\uparrow$ | F | ${ }_{1}$ | $\uparrow$ | \% |
| Traffic Volume (vph) | 47 | 543 | 102 | 16 | 184 | 6 | 90 | 386 | 44 | 12 | 243 | 13 |
| Future Volume (vph) | 47 | 543 | 102 | 16 | 184 | 6 | 90 | 386 | 44 | 12 | 243 | 13 |
| Lane Group Flow (vph) | 48 | 560 | 105 | 16 | 190 | 6 | 93 | 398 | 45 | 12 | 251 | 13 |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 | 6 |  | 6 | 2 |  |  |
| Detector Phase | 4 | 4 | 4 | 8 | 8 | 8 | 6 | 6 | 6 | 2 | 2 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 5.0 | 5.0 | 5.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Minimum Split (s) | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split (s) | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split (\%) | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 49.6\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% | 50.4\% |
| Yellow Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | None | None | Max | Max | Max | Max | Max | Max |
| v/c Ratio | 0.11 | 0.83 | 0.17 | 0.10 | 0.28 | 0.01 | 0.19 | 0.51 | 0.07 | 0.03 | 0.32 | 0.02 |
| Control Delay | 13.8 | 30.2 | 4.0 | 15.0 | 15.4 | 0.0 | 14.6 | 17.6 | 2.7 | 13.0 | 15.0 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 13.8 | 30.2 | 4.0 | 15.0 | 15.4 | 0.0 | 14.6 | 17.6 | 2.7 | 13.0 | 15.0 | 0.1 |
| Queue Length 50th (m) | 3.7 | 57.9 | 0.0 | 1.2 | 15.4 | 0.0 | 7.4 | 36.5 | 0.0 | 0.9 | 21.0 | 0.0 |
| Queue Length 95th ( m ) | 9.6 | \#96.2 | 7.9 | 4.9 | 28.2 | 0.0 | 16.4 | 60.0 | 3.5 | 3.8 | 36.8 | 0.0 |
| Internal Link Dist (m) |  | 1347.2 |  |  | 145.4 |  |  | 1762.3 |  |  | 135.7 |  |
| Turn Bay Length ( m ) | 100.0 |  | 50.0 | 143.0 |  | 58.0 | 142.0 |  | 48.0 | 45.0 |  | 40.0 |
| Base Capacity (vph) | 506 | 785 | 715 | 178 | 785 | 717 | 477 | 785 | 668 | 369 | 785 | 712 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Reduced v/c Ratio | 0.09 | 0.71 | 0.15 | 0.09 | 0.24 | 0.01 | 0.19 | 0.51 | 0.07 | 0.03 | 0.32 | 0.02 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 66.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 63.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 70 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type:SemiAct-Uncorrd two cycles. <br> \# 5 tht percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |  |



| AMandPM_Existing_20220426.syn | Synchro 11 Report |
| :--- | ---: |
| R.J. Burnside \& Associates | $05 / 20 / 2022$ - Page 5 |

HCM Signalized Intersection Capacity Analysis 3: Kennedy Road \& Elgin Mills Road East/Elgin Mills Road Existing 3. Kennedy Road \& Elgin Milis Road East/Elgin Mills Road Lost Time Adjustments


| Future Volume (vph) | 47 | 543 | 102 | 16 | 184 | 6 | 90 | 386 | 44 | 12 | 243 | 13 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| ost time | 7.0 | 7.0 | 7.0 | 7.0 |  | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |  |


| Total Lost time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Util. Factor | 1.00 | , 0 | 1.00 | , 0 | 1.00 | 1.00 | 1.00 | 1.00 | 00 | 1.00 | 1.00 |  |


| ane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Frt | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |


| Flt Protected | 1.05 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

atd. Flow (prot) |  | 1825 | 1902 | 1585 | 1825 | 1902 | 1633 | 1825 | 1902 | 1526 | 1825 | 1902 | 1633 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

| FIt Permitted | 0.64 | 1.00 | 1.00 | 0.23 | 1.00 | 1.00 | 0.60 | 1.00 | 1.00 | 0.47 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | Peak-hour factor, PHF | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 48 | 560 | 105 | 16 | 190 | 6 | 93 | 398 | 45 | 12 | 251 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Adj. Flow (vph) | 0 | 0 | 67 | 0 | 0 | 4 | 0 | 0 | 26 | 0 | 0 |


| Heavy Vehicles (\%) | $0 \%$ | $1 \%$ | $3 \%$ | $0 \%$ | $1 \%$ | $0 \%$ | $0 \%$ | $1 \%$ | $7 \%$ | $0 \%$ | $1 \%$ | 0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |

Protected Phases 4

| Permitted Phases | 4 | 4 | 8 | 8 | 6 | 6 | 2 | 6 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Actuated Green, G (s) | 22.6 | 22.6 | 22.6 | 22.6 | 22.6 | 22.6 | 26.1 | 26.1 | 26.1 | 26.1 | 26.1 | 26.1 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Effective Green, $\mathrm{g}(\mathrm{s})$ | 22.6 | 22.6 | 22.6 | 22.6 | 22.6 | 22.6 | 26.1 | 26.1 | 26.1 | 26.1 | 26.1 | 26.1 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Actuated $\operatorname{g} / \mathrm{C}$ Ratio | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 | 0.41 | 0.41 | 0.41 | 0.41 | 0.41 | 0.41 |


| Actuated g/C Ratio | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 | 0.36 | 0.41 | 0.41 | 0.41 | 0.41 | 0.41 | 0.41 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Clearance Time (s) | 7.0 | 70 | 7.0 | 7.0 | 70 | 70 | 75 | 75 | 75 | 75 | 75 | 7.5 |


|  | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 438 | 6.0 | 566 | 155 | 680 | 583 | 478 | 785 | 630 | 370 | 785 | 67 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Railo Prol v/s Ratio Perm Ratio
 $\begin{array}{llll}0.11 & 0.82 & 0.07 & 0.04 \\ 136 & 10.5 & 0.10\end{array}$

|  | 13.6 |
| :--- | :--- |
| Progression Factor | 1.00 |


|  | 1.00 |
| :--- | :--- |
| crogremsiontal |  |

mental Delay, d2
devel of Service
Level of Service
Approach Delay

| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 200 Control Delay | 18.3 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.65 |  | 14.5 |
| Actuated Cycle Length (s) | 63.2 | Sum of lost time (s) | F |
| Intersection Capacity Utilization | $93.9 \%$ | ICU Level of Service |  |
| Analysis Period (min) | 15 |  |  |

c Critical Lane Group

Timings
4：Kennedy Road \＆Major Mackenzie Drive Lost Time Adjustments


AMandPM＿Existing＿20220426．syn Synchro 11 Repor

R．J．Burnside \＆Associates

|  | $\dagger$ | $\rightarrow$ | 7 | $\checkmark$ | $\leftarrow$ | 4 | 4 | 4 | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | 个 $\uparrow$ | F | \％ | ¢ $\uparrow$ | 「 | 7 | 个个 | 7 | 7 | $\uparrow \uparrow$ | F |
| Traffic Volume（vph） | 37 | 1858 | 233 | 81 | 773 | 32 | 218 | 323 | 160 | 81 | 244 | 26 |
| Future Volume（vph） | 37 | 1858 | 233 | 81 | 773 | 32 | 218 | 323 | 160 | 81 | 244 | 26 |

$\left.\begin{array}{lrrrrrrrrrrrr} & 1920\end{array}\right)$

|  | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Total Lost time（s） | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Lane Util．Factor |  |  |  |  |  |  |  | 0 | 1.0 |  |  |  |


|  | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.95 | 1.00 | 1.00 | 0.95 | 1.09 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Frpb，ped／bikes | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 | 0.99 |


|  | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 | 0.98 | 1.00 | 1.00 | 0.99 | 1.00 | 1.00 | 0.99 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Satd．Flow（prot） | 1825 | 3650 | 1612 | 1755 | 3614 | 1597 | 1824 | 3650 | 1612 | 1824 | 3614 | 1612 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 0.32 | 1.00 | 1.00 | 0.07 | 1.00 | 1.00 | 0.47 | 1.00 | 1.00 | 0.55 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | 614 | 3650 | 1612 | 120 | 3614 | 1597 | 897 | 3650 | 1612 | 1061 | 3614 | 1612 |


| Peak－hour factor，PHF | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 38 | 1896 | 238 | 83 | 789 | 33 | 222 | 330 | 163 | 83 | 249 |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Adj．Flow（vph） | 0 | 54 | 0 | 0 | 15 | 0 | 0 | 102 | 0 | 0 |  |  |


| Lane Group Flow（vph） | 38 | 1896 | 184 | 83 | 789 | 18 | 222 | 330 | 61 | 83 | 249 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Confl．Peds．（\＃hr） | 1 |  | 1 | 1 |  | 1 | 1 |  | 1 | 1 |  |


| leavy Vehicles（\％） | $0 \%$ | $0 \%$ | $0 \%$ | $4 \%$ | $1 \%$ | $0 \%$ | $0 \%$ | $0 \%$ | $0 \%$ | $0 \%$ | $1 \%$ | $0 \%$ |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Turn Type | $\mathrm{pm}+\mathrm{pt}$ | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |

$$
\begin{aligned}
& \text { Turn Type } \\
& \text { Orotected }
\end{aligned}
$$

$\qquad$

| Protected Phases | 1 | 6 | 6 | 5 | 2 | 2 | 8 | 8 | 8 | 4 | 4 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | Actuated Green， $\mathrm{G}(\mathrm{s})$ | 63.7 | 59.6 | 59.6 | 67.3 | 61.4 | 61.4 | 26.7 | 26.7 | 26.7 | 14.7 | 14.7 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Effective Green， $\mathrm{g}(\mathrm{s})$ | 63.7 | 59.6 | 59.6 | 67.3 | 61.4 | 61.4 | 26.7 | 26.7 | 26.7 | 14.7 | 14.7 | 14.7 |


| Actuated g／C Ratio | 0.57 | 0.54 | 0.54 | 0.61 | 0.55 | 0.55 | 0.24 | 0.24 | 0.24 | 0.13 | 0.13 | 0.13 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Actuated g／C Ratio | 0.5 | 0.54 | 0.54 | 0.6 | 0.55 | 0.55 | 0.24 | 0.24 | 0.24 | .13 | .13 |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Clearance Time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Vehicle Extension（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Grp Cap（vph） | 396 | 1956 | 863 | 159 | 1995 | 881 | 282 | 876 | 387 | 140 | 477 | 213 |
| v／s Ratio Prot | 0.00 | $c 0.52$ |  | $c 0.03$ | 0.22 |  | $c 0.0$ | 0.09 |  |  | 0.07 |  |
| v／s Ratio Perm | 0.05 |  | 0.11 | 0.29 |  | 0.01 | $c 0.13$ |  | 0.04 | 0.08 |  | 0.00 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| R Ratio Perm | 0.05 | 0.11 | 0.29 | 0.40 | 0.01 | $c 0.13$ |  | 0.04 | 0.08 | 0.52 | 0.00 |  |
|  | 0.10 | 0.97 | 0.21 | 0.52 | 0.40 | 0.02 | 0.79 | 0.38 | 0.16 | 0.59 | 0.52 | 0.02 |


| Uniform Delay，d1 | 10.5 | 24.9 | 13.5 | 24.6 | 14.3 | 11.3 | 38.2 | 35.3 | 33.4 | 45.4 | 45.0 | 42.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 0.1 | 14.3 | 0.6 | 3.1 | 0.6 | 0.0 | 13.5 | 0.3 | 0.2 | 6.6 | 1.0 | 0.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | 10.6 | 39.2 | 14.1 | 27.2 | 14.9 | 11.3 | 51.7 | 35.6 | 33.6 | 52.0 | 46.0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


| Level of Service | B | D | B | C | B | B | D | D | C | D | D |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Approach Delay（s） |  | 36.0 |  |  | 15.9 |  |  | 40.1 |  |  | 47.1 |


| pproach LOS | D | B | D | D |
| :---: | :---: | :---: | :---: | :---: |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Contral Delay | 33.3 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.92 |  | 23.0 |
| Actuated Cycle Length（s） | 111.2 | Sum of lost time（s） | F |
| Intersection Capacity Utilization | $97.3 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |

Arsection Cacitiy Utilization 97．3\％

CU Level of Servic
c Critical Lane Group
[The Difference is our People]

## Appendix F

## York Region's Transportation Model Refinements EMME Plots











 BURNSIDE
[The Difference is our People]

Appendix G
2041 Future Conditions Synchro Reports

|  | 4 |  | 7 |  | 4 | $\uparrow$ |  | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ | \% | F | \% | $\uparrow$ | 「 | ${ }^{7}$ | $\uparrow$ | 「 |
| Traffic Volume (vph) | 24 | 606 | 176 | 1408 | 101 | 244 | 14 | 7 | 1651 | 100 |
| Future Volume (vph) | 24 | 606 | 176 | 1408 | 101 | 244 | 14 | 7 | 1651 | 100 |
| Lane Group Flow (vph) | 28 | 906 | 205 | 1657 | 117 | 284 | 16 | 8 | 1920 | 116 |
| Turn Type | Perm | NA | Perm | NA | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  | 8 |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 8 |  | 6 |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 10.0 | 10.0 | 10.0 | 10.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| Minimum Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (\%) | 52.2\% | 52.2\% | 52.2\% | 52.2\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% |
| Yellow Time (s) | 4.5 | 4.5 | 4.5 | 4.5 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 2.5 | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | -2.5 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 4.5 | 7.0 |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | Max | Max | Max | Max | Max | Max |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.26 | 1.25 | 1.88 | 2.11 | 1.04 | 0.41 | 0.02 | 0.02 | 2.49 | 0.18 |
| Control Delay | 20.8 | 147.7 | 450.0 | 522.3 | 127.2 | 17.8 | 0.1 | 13.6 | 690.7 | 8.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 20.8 | 147.7 | 450.0 | 522.3 | 127.2 | 17.8 | 0.1 | 13.6 | 690.7 | 8.1 |
| Queue Length 50th (m) | 2.2 | ~146.3 | $\sim 40.0$ | ~343.6 | ~16.2 | 25.5 | 0.0 | 0.6 | $\sim 408.3$ | 4.0 |
| Queue Length 95th (m) | 8.0 | \#197.1 | \#60.5 | \#395.8 | \#41.5 | 41.3 | 0.0 | 2.8 | \#457.5 | 12.3 |
| Internal Link Dist ( m ) |  | 180.4 |  | 543.1 |  | 1650.0 |  |  | 238.4 |  |
| Turn Bay Length ( m ) | 70.0 |  | 80.0 |  | 125.0 |  | 30.0 | 100.0 |  | 30.0 |
| Base Capacity (vph) | 106 | 723 | 109 | 786 | 112 | 689 | 650 | 404 | 772 | 638 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.26 | 1.25 | 1.88 | 2.11 | 1.04 | 0.41 | 0.02 | 0.02 | 2.49 | 0.18 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |

Cycle Length: 67
Actuated Cycle Length: 67
Natural Cycle: 110
Control Type: Semi Act-Uncoord
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 1: Warden Avenue \& Elgin Mills Road East



2：Warden Avenue \＆Major Mackenzie Drive

|  | $\Rightarrow$ |  |  | 7 |  |  | 4 | 4 | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 个个 | F | \％ | 个个 | $\overline{7}$ | ${ }^{7}$ | 个个 | F | ${ }^{7}$ | $\uparrow$ | F |
| Trafic Volume（vph） | 1 | 677 | 140 | 344 | 2037 | 24 | 65 | 239 | 64 | 38 | 1628 | 113 |
| Future Volume（vph） | 1 | 677 | 140 | 344 | 2037 | 24 | 65 | 239 | 64 | 38 | 1628 | 113 |
| Lane Group Flow（vph） | 1 | 728 | 151 | 370 | 2190 | 26 | 70 | 257 | 69 | 41 | 1751 | 122 |
| Turn Type | Perm | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 6 |  | 5 | 2 |  | 3 | 8 |  |  | 4 |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 6 | 6 | 6 | 5 | 2 | 2 | 3 | 8 | 8 | 4 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（ $s$ ） | 34.0 | 34.0 | 34.0 | 7.0 | 34.0 | 34.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Minimum Split（s） | 41.5 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 41.5 | 41.5 | 41.5 |
| Total Split（s） | 43.0 | 43.0 | 43.0 | 25.0 | 68.0 | 68.0 | 12.0 | 62.0 | 62.0 | 50.0 | 50.0 | 50.0 |
| Total Split（\％） | 33．1\％ | 33．1\％ | 33．1\％ | 19．2\％ | 52．3\％ | 52．3\％ | 9．2\％ | 47．7\％ | 47．7\％ | 38．5\％ | 38．5\％ | 38．5\％ |
| Yellow Time（s） | 5.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |
| All－Red Time（s） | 2.5 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | －2．5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 7.5 | 7.5 | 7.5 | 4.0 | 5.0 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag | Lag | Lag | Lag | Lead |  |  | Lead |  |  | Lag | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes |  |  | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | Max | Max | Max | None | Max | Max | None | None | None | None | None | None |
| v／c Ratio | 0.02 | 0.75 | 0.27 | 0.93 | 1.22 | 0.03 | 0.43 | 0.18 | 0.11 | 0.12 | 2.76 | 0.20 |
| Control Delay | 36.0 | 48.1 | 6.9 | 57.5 | 136.8 | 0.1 | 29.2 | 24.4 | 5.4 | 32.0 | 815.9 | 8.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 36.0 | 48.1 | 6.9 | 57.5 | 136.8 | 0.1 | 29.2 | 24.4 | 5.4 | 32.0 | 815.9 | 8.7 |
| Queue Length 50th（m） | 0.2 | 90.7 | 0.0 | 64.6 | ～368．1 | 0.0 | 10.2 | 21.4 | 0.0 | 7.4 | ～766．4 | 3.0 |
| Queue Length 95th（m） | 1.8 | 113.7 | 15.8 | \＃123．9 | \＃409．0 | 0.0 | 19.3 | 30.5 | 8.6 | 16.2 | \＃846．8 | 16.6 |
| Internal Link Dist（ $m$ ） |  | 203.6 |  |  | 501.9 |  |  | 120.0 |  |  | 316.9 |  |
| Turn Bay Length（ m ） | 58.0 |  | 90.0 | 138.0 |  | 111.0 | 48.0 |  | 32.0 | 146.0 |  | 80.0 |
| Base Capacity（vph） | 60 | 969 | 550 | 401 | 1788 | 812 | 167 | 1502 | 633 | 345 | 634 | 615 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.02 | 0.75 | 0.27 | 0.92 | 1.22 | 0.03 | 0.42 | 0.17 | 0.11 | 0.12 | 2.76 | 0.20 |

Cycle Length： 130
Actuated Cycle Length： 127.5
Natural Cycle： 145
Control Type：Semi Act－Uncoord
～Volume exceeds capacity，queue is theoretically infinite． Queue shown is maximum after two cycles．
\＃95th percentile volume exceeds capacity，queue may be longer．
Queue shown is maximum after two cycles．
Splits and Phases：2：Warden Avenue \＆Major Mackenzie Drive



3：Kennedy Road \＆Elgin Mills Road East／Elgin Mills Road

|  | 4 |  |  |  |  |  | 4 | ¢ |  | ＊ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 4 | T | ${ }^{*}$ | 个 | 「 | ${ }^{7}$ | 4 | 「 | ${ }^{*}$ | 4 | 7 |
| Traffic Volume（vph） | 14 | 364 | 110 | 81 | 1218 | 14 | 149 | 375 | 9 | 17 | 922 | 106 |
| Future Volume（vph） | 14 | 364 | 110 | 81 | 1218 | 14 | 149 | 375 | 9 | 17 | 922 | 106 |
| Lane Group Flow（vph） | 16 | 414 | 125 | 92 | 1384 | 16 | 169 | 426 | 10 | 19 | 1048 | 120 |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 | 6 |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 4 | 8 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Minimum Split（s） | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split（s） | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split（\％） | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ |
| Yellow Time（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All－Red Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | －2．5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 5.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | None | None | Max | Max | Max | Max | Max | Max |
| v／c Ratio | 0.14 | 0.57 | 0.18 | 0.33 | 1.92 | 0.02 | 1.47 | 0.58 | 0.01 | 0.06 | 1.40 | 0.18 |
| Control Delay | 16.9 | 19.6 | 7.2 | 18.5 | 438.2 | 0.1 | 276.2 | 20.0 | 0.0 | 13.5 | 208.7 | 7.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 16.9 | 19.6 | 7.2 | 18.5 | 438.2 | 0.1 | 276.2 | 20.0 | 0.0 | 13.5 | 208.7 | 7.7 |
| Queue Length 50th（m） | 1.3 | 38.8 | 3.9 | 7.7 | $\sim 270.2$ | 0.0 | ～29．4 | 40.2 | 0.0 | 1.4 | ～177．6 | 4.1 |
| Queue Length 95th（m） | 5.0 | 61.5 | 12.5 | 18.0 | \＃330．9 | 0.0 | \＃48．1 | 63.6 | 0.0 | 5.0 | \＃235．4 | 12.7 |
| Internal Link Dist（m） |  | 1347.2 |  |  | 145.4 |  |  | 1762.3 |  |  | 135.7 |  |
| Turn Bay Length（m） | 100.0 |  | 50.0 | 143.0 |  | 58.0 | 142.0 |  | 48.0 | 45.0 |  | 40.0 |
| Base Capacity（vph） | 115 | 729 | 677 | 275 | 722 | 683 | 115 | 729 | 678 | 310 | 751 | 672 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.14 | 0.57 | 0.18 | 0.33 | 1.92 | 0.02 | 1.47 | 0.58 | 0.01 | 0.06 | 1.40 | 0.18 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 66.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 66.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 80 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Semi Act－Uncoord |  |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ Volume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：$\quad$ ：Kennedy Road \＆Elgin Mills Road East／Elgin Mills Road



|  | 4 | $\rightarrow$ | ＊ | 7 | $\checkmark$ | 4 | 4 | $\uparrow$ | 7 | － | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个 $\uparrow$ | 「 | \％ | 4 4 | 「 | \％ | ¢ $\uparrow$ | 「 | \％ | 44 | 「 |
| Trafic Volume（vph） | 35 | 633 | 249 | 179 | 2048 | 173 | 275 | 405 | 100 | 90 | 791 | 155 |
| Future Volume（vph） | 35 | 633 | 249 | 179 | 2048 | 173 | 275 | 405 | 100 | 90 | 791 | 155 |
| Lane Group Flow（vph） | 38 | 696 | 274 | 197 | 2251 | 190 | 302 | 445 | 110 | 99 | 869 | 170 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| Protected Phases | 1 | 6 |  | 5 | 2 |  | 3 | 8 |  |  | 4 |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 1 | 6 | 6 | 5 | 2 |  | 3 | 8 | 8 | 4 | 4 | 4 |

Switch Phase

| Minimum Initial（ s ） | 7.0 | 15.0 | 15.0 | 7.0 | 15.0 | 15.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Minimum Split（s） | 11.0 | 37.5 | 37.5 | 11.0 | 37.5 | 37.5 | 11.0 | 38.5 | 38.5 | 38.5 | 38.5 | 38.5 |
| Total Split（s） | 13.0 | 63.0 | 63.0 | 13.0 | 63.0 | 63.0 | 15.0 | 54.0 | 54.0 | 39.0 | 39.0 | 39.0 |
| Total Split（\％） | 10．0\％ | 48．5\％ | 48．5\％ | 10．0\％ | 48．5\％ | 48．5\％ | 11．5\％ | 41．5\％ | 41．5\％ | 30．0\％ | 30．0\％ | 30．0\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All－Red Time（s） | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | －2．5 | 0.0 | －2．0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 5.0 | 7.5 | 2.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead |  |  | Lag | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | None | Max | Max | None | Max | Max | None | None | None | None | None | None |
| v／c Ratio | 0.24 | 0.46 | 0.33 | 0.52 | 1.31 | 0.24 | 1.25 | 0.35 | 0.17 | 0.44 | 1.00 | 0.35 |
| Control Delay | 17.2 | 27.8 | 3.7 | 20.1 | 173.7 | 6.2 | 171.0 | 31.6 | 5.6 | 48.9 | 80.2 | 13.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 17.2 | 27.8 | 3.7 | 20.1 | 173.7 | 6.2 | 171.0 | 31.6 | 5.6 | 48.9 | 80.2 | 13.7 |
| Queue Length 50th（m） | 4.3 | 65.9 | 0.0 | 24.3 | $\sim 396.9$ | 4.9 | $\sim 79.1$ | 43.7 | 0.0 | 21.6 | $\sim 118.3$ | 8.0 |
| Queue Length 95th（m） | 9.4 | 82.7 | 15.6 | 37.5 | \＃442．2 | 18.9 | \＃135．8 | 57.7 | 12.2 | 39.6 | \＃162．0 | 27.1 |
| Internal Link Dist（ m ） |  | 1479.7 |  |  | 222.6 |  |  | 139.6 |  |  | 212.5 |  |
| Turn Bay Length（ m ） | 56.0 |  | 66.0 | 60.0 |  | 145.0 | 120.0 |  | 55.0 | 60.0 |  | 65.0 |
| Base Capacity（vph） | 180 | 1513 | 843 | 382 | 1720 | 791 | 242 | 1280 | 649 | 227 | 867 | 484 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.21 | 0.46 | 0.33 | 0.52 | 1.31 | 0.24 | 1.25 | 0.35 | 0.17 | 0.44 | 1.00 | 0.35 |

## Intersection Summary

Cycle Length： 130
Actuated Cycle Length： 130
Natural Cycle： 150
Control Type：Semi Act－Uncoord
～Volume exceeds capacity，queue is theoretically infinite． Queue shown is maximum after two cycles．
\＃95th percentile volume exceeds capacity，queue may be longer．
Queue shown is maximum after two cycles．
Splits and Phases：4：Kennedy Road \＆Major Mackenzie Drive


AMandPM＿Future．syn

c Critical Lane Group

|  |  | $\rightarrow$ |  |  |  |  | $p$ |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ | \% | $\uparrow$ | ${ }^{7}$ | $\uparrow$ | 7 | 7 | $\uparrow$ | 7 |
| Trafic Volume (vph) | 50 | 1156 | 23 | 469 | 204 | 1240 | 155 | 15 | 251 | 22 |
| Future Volume (vph) | 50 | 1156 | 23 | 469 | 204 | 1240 | 155 | 15 | 251 | 22 |
| Lane Group Flow (vph) | 58 | 1464 | 27 | 561 | 237 | 1442 | 180 | 17 | 292 | 26 |
| Turn Type | Perm | NA | Perm | NA | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  | 8 |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 8 |  | 6 |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial ( s ) | 10.0 | 10.0 | 10.0 | 10.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| Minimum Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (s) | 35.0 | 35.0 | 35.0 | 35.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 | 32.0 |
| Total Split (\%) | 52.2\% | 52.2\% | 52.2\% | 52.2\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% | 47.8\% |
| Yellow Time (s) | 4.5 | 4.5 | 4.5 | 4.5 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All-Red Time (s) | 2.5 | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | -2.5 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 4.5 | 7.0 | 7.0 | 7.0 | 7.0 |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | Max | Max | Max | Max | Max | Max |
| v/c Ratio | 0.28 | 1.98 | 0.25 | 0.71 | 0.61 | 1.90 | 0.28 | 0.15 | 0.42 | 0.04 |
| Control Delay | 17.4 | 465.1 | 19.9 | 22.3 | 25.1 | 431.4 | 10.7 | 18.1 | 17.9 | 0.9 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 17.4 | 465.1 | 19.9 | 22.3 | 25.1 | 431.4 | 10.7 | 18.1 | 17.9 | 0.9 |
| Queue Length 50th (m) | 4.6 | $\sim 296.5$ | 2.1 | 55.5 | 23.3 | $\sim 283.0$ | 9.4 | 1.4 | 26.3 | 0.0 |
| Queue Length 95th (m) | 12.1 | \#349.2 | 7.6 | 82.5 | 42.5 | \#334.1 | 20.2 | 5.4 | 42.2 | 0.8 |
| Internal Link Dist ( m ) |  | 180.4 |  | 543.1 |  | 1650.0 |  |  | 238.4 |  |
| Turn Bay Length ( m ) | 70.0 |  | 80.0 |  | 125.0 |  | 30.0 | 100.0 |  | 30.0 |
| Base Capacity (vph) | 208 | 740 | 109 | 786 | 389 | 758 | 650 | 114 | 702 | 638 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.28 | 1.98 | 0.25 | 0.71 | 0.61 | 1.90 | 0.28 | 0.15 | 0.42 | 0.04 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 67 |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 67 |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 150 |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |
| ~ Volume exceeds capacity, queue is theoretically infinite. |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 1: Warden Avenue \& Elgin Mills Road East



|  | $\rangle$ |  |  | $\dagger$ |  |  | 4 | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | ¢4 | 7 | ${ }^{*}$ | ¢4 | 「 | ${ }^{*}$ | ¢4 | $\overline{7}$ | ${ }^{7}$ | 4 | F |
| Traffic Volume (vph) | 42 | 1970 | 126 | 109 | 740 | 37 | 210 | 1584 | 330 | 35 | 293 | 21 |
| Future Volume (vph) | 42 | 1970 | 126 | 109 | 740 | 37 | 210 | 1584 | 330 | 35 | 293 | 21 |
| Lane Group Flow (vph) | 45 | 2118 | 135 | 117 | 796 | 40 | 226 | 1703 | 355 | 38 | 315 | 23 |
| Turn Type | Perm | NA | Perm | pm+pt | NA | Perm | pm+pt | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 6 |  | 5 | 2 |  | 3 | 8 |  |  | , |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 6 | 6 | 6 | 5 | 2 | 2 | 3 | 8 | 8 | 4 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 34.0 | 34.0 | 34.0 | 7.0 | 34.0 | 34.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Minimum Split (s) | 41.5 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 11.0 | 41.5 | 41.5 | 41.5 | 41.5 | 41.5 |
| Total Split (s) | 63.0 | 63.0 | 63.0 | 12.0 | 75.0 | 75.0 | 12.0 | 55.0 | 55.0 | 43.0 | 43.0 | 43.0 |
| Total Split (\%) | 48.5\% | 48.5\% | 48.5\% | 9.2\% | 57.7\% | 57.7\% | 9.2\% | 42.3\% | 42.3\% | 33.1\% | 33.1\% | 33.1\% |
| Yellow Time (s) | 5.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |
| All-Red Time (s) | 2.5 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lost Time Adjust (s) | 0.0 | -2.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 7.5 | 5.0 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead/Lag | Lag | Lag | Lag | Lead |  |  | Lead |  |  | Lag | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes |  |  | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | Max | Max | Max | None | Max | Max | None | None | None | None | None | None |
| v/c Ratio | 0.16 | 1.38 | 0.18 | 0.70 | 0.42 | 0.05 | 0.74 | 1.33 | 0.63 | 0.72 | 0.61 | 0.04 |
| Control Delay | 24.8 | 204.0 | 7.3 | 43.0 | 20.1 | 1.0 | 45.4 | 187.8 | 30.7 | 106.0 | 47.0 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 24.8 | 204.0 | 7.3 | 43.0 | 20.1 | 1.0 | 45.4 | 187.8 | 30.7 | 106.0 | 47.0 | 0.1 |
| Queue Length 50th (m) | 7.0 | ~378.0 | 4.4 | 14.0 | 64.1 | 0.0 | 40.1 | $\sim 297.4$ | 56.0 | 8.8 | 70.5 | 0.0 |
| Queue Length 95th (m) | 15.6 | \#419.0 | 16.6 | \#39.5 | 79.3 | 1.8 | \#64.4 | \#339.6 | 89.8 | \#29.3 | 101.4 | 0.0 |
| Internal Link Dist (m) |  | 203.6 |  |  | 501.9 |  |  | 120.0 |  |  | 316.9 |  |
| Turn Bay Length ( m ) | 58.0 |  | 90.0 | 138.0 |  | 111.0 | 48.0 |  | 32.0 | 146.0 |  | 80.0 |
| Base Capacity (vph) | 288 | 1540 | 732 | 169 | 1876 | 882 | 307 | 1282 | 562 | 53 | 519 | 522 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.16 | 1.38 | 0.18 | 0.69 | 0.42 | 0.05 | 0.74 | 1.33 | 0.63 | 0.72 | 0.61 | 0.04 |

Cycle Length: 130
Actuated Cycle Length: 130
Natural Cycle: 145
Control Type: Semi Act-Uncoord
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 2: Warden Avenue \& Major Mackenzie Drive



3：Kennedy Road \＆Elgin Mills Road East／Elgin Mills Road

|  | $\rangle$ |  |  | $\checkmark$ |  |  | 4 | $\uparrow$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | $\uparrow$ | 「 | ${ }^{4}$ | $\uparrow$ | 「 | \％ | $\uparrow$ | 「 | \％ | $\uparrow$ | F |
| Traffic Volume（vph） | 59 | 1123 | 131 | 21 | 380 | 7 | 115 | 632 | 56 | 15 | 398 | 16 |
| Future Volume（vph） | 59 | 1123 | 131 | 21 | 380 | 7 | 115 | 632 | 56 | 15 | 398 | 16 |
| Lane Group Flow（vph） | 67 | 1276 | 149 | 24 | 432 | 8 | 131 | 718 | 64 | 17 | 452 | 18 |
| Turn Type | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases |  | 4 |  |  | 8 |  |  | 6 |  |  | 2 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 | 6 |  | 6 | 2 |  | 2 |
| Detector Phase | 4 | 4 | 4 | 8 | 8 | 8 | 6 | 6 | 6 | 2 | 2 | 2 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 10.0 | 10.0 | 10.0 | 5.0 | 5.0 | 5.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Minimum Split（s） | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split（s） | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.0 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 | 33.5 |
| Total Split（\％） | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 49．6\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ | 50．4\％ |
| Yellow Time（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All－Red Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None | None | None | None | Max | Max | Max | Max | Max | Max |
| v／c Ratio | 0.22 | 1.75 | 0.22 | 0.24 | 0.60 | 0.01 | 0.45 | 0.98 | 0.09 | 0.15 | 0.60 | 0.03 |
| Control Delay | 16.1 | 364.6 | 7.9 | 21.0 | 20.4 | 0.0 | 21.3 | 52.9 | 4.2 | 17.1 | 20.3 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 16.1 | 364.6 | 7.9 | 21.0 | 20.4 | 0.0 | 21.3 | 52.9 | 4.2 | 17.1 | 20.3 | 0.1 |
| Queue Length 50th（m） | 5.4 | $\sim 240.7$ | 5.3 | 1.9 | 41.1 | 0.0 | 11.6 | 85.1 | 0.0 | 1.3 | 43.1 | 0.0 |
| Queue Length 95th（m） | 13.2 | \＃300．7 | 15.0 | 7.4 | 65.0 | 0.0 | 25.6 | \＃145．9 | 5.8 | 5.4 | 67.3 | 0.0 |
| Internal Link Dist（ m ） |  | 1347.2 |  |  | 145.4 |  |  | 1762.3 |  |  | 135.7 |  |
| Turn Bay Length（ m ） | 100.0 |  | 50.0 | 143.0 |  | 58.0 | 142.0 |  | 48.0 | 45.0 |  | 40.0 |
| Base Capacity（vph） | 298 | 729 | 680 | 101 | 722 | 683 | 289 | 729 | 678 | 115 | 751 | 672 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.22 | 1.75 | 0.22 | 0.24 | 0.60 | 0.01 | 0.45 | 0.98 | 0.09 | 0.15 | 0.60 | 0.03 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 66.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 66.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 150 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Semi Act－Uncoord |  |  |  |  |  |  |  |  |  |  |  |  |
| ～Volume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer．Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：$\quad$ ：Kennedy Road \＆Elgin Mills Road East／Elgin Mills Road



|  | $\rangle$ |  |  | 7 |  | 4 | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 个4 | \％ | \％ | 个4 | 「 | \％ | 个4 | 「 | \％ | 个4 | F |
| Traffic Volume（vph） | 47 | 2053 | 245 | 85 | 854 | 41 | 229 | 529 | 168 | 104 | 400 | 33 |
| Future Volume（vph） | 47 | 2053 | 245 | 85 | 854 | 41 | 229 | 529 | 168 | 104 | 400 | 33 |
| Lane Group Flow（vph） | 52 | 2256 | 269 | 93 | 938 | 45 | 252 | 581 | 185 | 114 | 440 | 36 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Perm | NA | Perm |
| Protected Phases | 1 | 6 |  | 5 | 2 |  | 3 | 8 |  |  | 4 |  |
| Permitted Phases | 6 |  | 6 | 2 |  | 2 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 1 | 6 | 6 | 5 | 2 | 2 | 3 | 8 | 8 | 4 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 7.0 | 15.0 | 15.0 | 7.0 | 15.0 | 15.0 | 7.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Minimum Split（s） | 11.0 | 37.5 | 37.5 | 11.0 | 37.5 | 37.5 | 11.0 | 38.5 | 38.5 | 38.5 | 38.5 | 38.5 |
| Total Split（s） | 12.0 | 66.0 | 66.0 | 12.0 | 66.0 | 66.0 | 12.0 | 52.0 | 52.0 | 40.0 | 40.0 | 40.0 |
| Total Split（\％） | 9．2\％ | 50．8\％ | 50．8\％ | 9．2\％ | 50．8\％ | 50．8\％ | 9．2\％ | 40．0\％ | 40．0\％ | 30．8\％ | 30．8\％ | 30．8\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| All－Red Time（s） | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 1.0 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 4.0 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead |  |  | Lag | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes |  |  | Yes | Yes | Yes |
| Recall Mode | None | Max | Max | None | Max | Max | None | None | None | None | None | None |
| v／c Ratio | 0.16 | 1.30 | 0.32 | 0.53 | 0.51 | 0.05 | 0.98 | 0.56 | 0.33 | 0.74 | 0.65 | 0.09 |
| Control Delay | 11.9 | 168.5 | 12.7 | 27.0 | 22.2 | 0.1 | 88.8 | 38.2 | 11.9 | 73.1 | 49.6 | 0.5 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 11.9 | 168.5 | 12.7 | 27.0 | 22.2 | 0.1 | 88.8 | 38.2 | 11.9 | 73.1 | 49.6 | 0.5 |
| Queue Length 50th（m） | 4.5 | $\sim 359.0$ | 20.7 | 8.3 | 76.7 | 0.0 | 47.1 | 61.3 | 9.2 | 25.6 | 51.0 | 0.0 |
| Queue Length 95th（m） | 11.5 | \＃448．8 | 44.9 | 24.9 | 112.5 | 0.0 | \＃92．4 | 78.3 | 26.5 | 46.1 | 67.4 | 0.0 |
| Internal Link Dist（m） |  | 1479.7 |  |  | 222.6 |  |  | 139.6 |  |  | 212.5 |  |
| Turn Bay Length（ $m$ ） | 56.0 |  | 66.0 | 60.0 |  | 145.0 | 120.0 |  | 55.0 | 60.0 |  | 65.0 |
| Base Capacity（vph） | 345 | 1734 | 843 | 184 | 1848 | 842 | 257 | 1332 | 684 | 224 | 973 | 510 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | ， | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.15 | 1.30 | 0.32 | 0.51 | 0.51 | 0.05 | 0.98 | 0.44 | 0.27 | 0.51 | 0.45 | 0.07 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 130
Actuated Cycle Length： 119.9
Natural Cycle： 150
Control Type：Semi Act－Uncoord
～Volume exceeds capacity，queue is theoretically infinite． Queue shown is maximum after two cycles．
\＃95th percentile volume exceeds capacity，queue may be longer．
Queue shown is maximum after two cycles．
Splits and Phases：4：Kennedy Road \＆Major Mackenzie Drive


c Critical Lane Group


